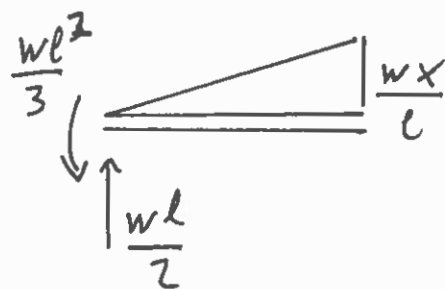


## Section A

$$1a) i) v_x = \frac{wx}{l}$$

$$ii) M = \frac{wl^2}{3} + \frac{wx^3}{6l} - \frac{wlx}{2}$$



$$EIv'' = M$$

$$EIv' = \frac{wl^2}{3}x + \frac{wx^4}{24l} - \frac{wlx^2}{4} + C_1$$

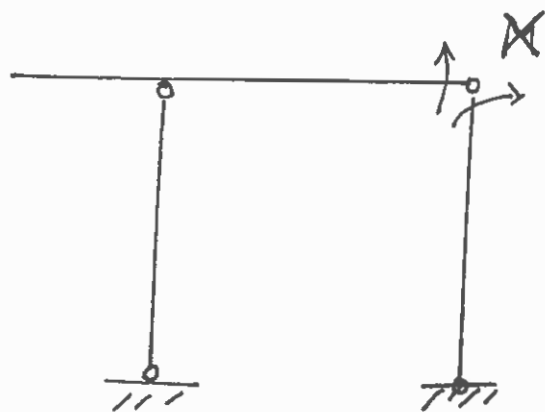
$$EIv = \frac{wl^2}{6}x^2 + \frac{wx^5}{120l} - \frac{wlx^3}{12} + C_1x + C_2$$

Boundary conditions:  $v(0) = 0$   $v'(0) = 0$

$$\Rightarrow EIv = \frac{wx^5}{120l} - \frac{wlx^3}{12} + \frac{wl^2x^2}{6}$$

$$EIv(l) = \frac{11wl^4}{120} \quad EIv'(l) = \frac{wl^3}{8}$$

b) i)



$$EI\theta_1 = \frac{l}{3} + l = \frac{4}{3}l$$

$$EI\theta_0 = \frac{wl^3}{8}$$

$$\Rightarrow X\theta_1 = \theta_0$$

$$X = \frac{3}{32}wl^2$$

Moment at C : 93.75 kNm

Axial forces

$$V_E = \frac{93.75}{10} = 9.375 \text{ kN}$$

$$V_D = -9.375 \text{ kN}$$

(2)

ii) Horizontal deflection

$$\delta_{H,A} = \frac{11wL^4}{120EI} - \frac{Ml^2}{2EI} = 45.83 - 23.48 = 22.40 \text{ mm}$$

Vertical deflection

$$\delta_{V,A} = \frac{Ml}{6EI} \cdot 5 = \frac{33.75 \cdot 5 \cdot 10}{6 \cdot 200 \cdot 10^6} = 3.3 \text{ mm}$$

iii) Deformation at B only, vertical and 100 mm

$$\theta_0 = \frac{0.1}{10} \quad EI\theta_1 = \frac{4}{3} l$$

$$\Rightarrow X = 0.01 \cdot \frac{3}{4l} EI = 150 \text{ kNm} \quad \text{Moment at C}$$

2a) i)

$$I = \pi r^3 t = 3.14 \cdot 10^{-5}$$

(3)

$$\sigma_A = \frac{M_y}{I} = \frac{25 \cdot 10^3 \cdot 0.1}{3.14 \cdot 10^{-5}} = -79.6 \text{ MPa}$$

$$\sigma_B = 0$$

ii) Due to torque

$$\tau_x = \frac{T}{2A_e t} = \frac{25 \cdot 10^3 \cdot 0.1}{2\pi \cdot 0.1^2 \cdot 0.01} = 3.98 \text{ MPa}$$

Due to shear stress

at A is zero

at B is

$$\tau_s = -\frac{SA\bar{y}}{I \cdot 2t} = \frac{25 \cdot 10^3 \cdot \pi \cdot 0.1 \cdot 0.01 \cdot 0.064}{3.14 \cdot 10^{-5} \cdot 0.02} = -7.96 \text{ MPa}$$

Total at A  $\tau = 3.98 \text{ MPa}$

at B  $\tau = -12 \text{ MPa}$

b) i) Mohr's circle at B  $\sigma_1 = -3.98^{\text{MPa}}$  and  $\sigma_2 = 3.98 \text{ MPa}$

Mohr's circle at A

$$\sigma_A = -79.6 \text{ MPa} \quad \tau = 3.98 \text{ MPa}$$

$$\sigma_1 = -80 \text{ MPa} \quad \sigma_2 = 0.2 \text{ MPa}$$

A is clearly critical, so using

$$\lambda^2 \left[ (\sigma_1 - \sigma_2)^2 + \tau_2^2 + \tau_1^2 \right] = 2Y^2$$

$$\lambda^2 [80.2^2 + 0.2^2 + 80^2] = 2.275$$

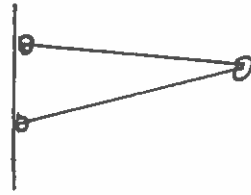
$$\Rightarrow \lambda = 3.41$$

ii) too high

iii) tapering tube



struts



box cross section etc.

.....

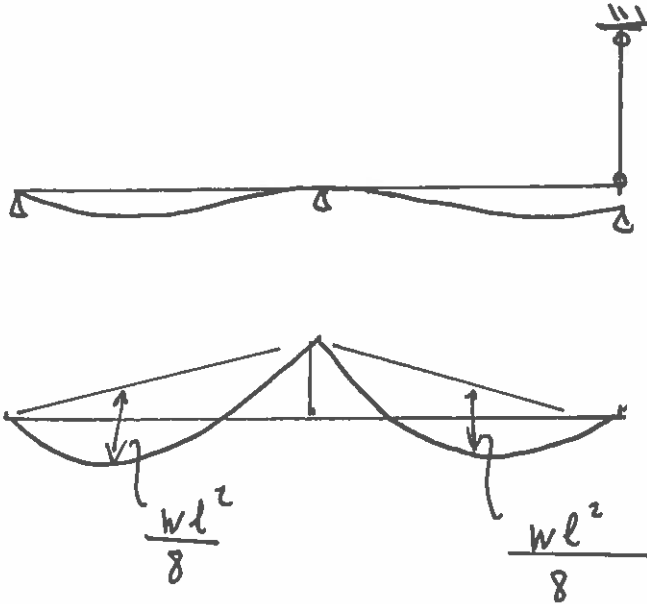
For instance tapering tube

$$\sigma = \frac{Mr}{2\pi r^3 t} = \frac{M}{2\pi r^2 t}$$

Need to reduce stress by a factor of  $\sim 4$ .

Increase radius by  $\sim 2$ .

3) a)

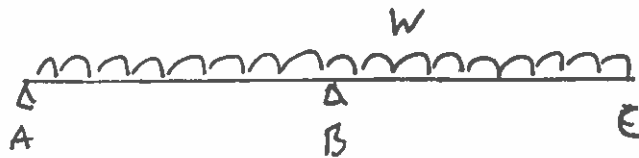


(5)

b) i) Number of redundancies is 1

- Remove support A
- Introduce pin at B
- Cut cable
- ...

ii)



rotation at B

$$\text{Moment at B: } M_B = \frac{ql^2}{2}$$

$$EI \theta_B = -\frac{wl^3}{24} + \frac{wl^3}{60} = \frac{wl^3}{8} \Rightarrow EI \delta_1 = \frac{wl^4}{8}$$

BC as cantilever

$$EI \delta_2 = \frac{wl^4}{8}$$

Total disp:

$$\delta = \delta_1 + \delta_2 = \frac{wl^4}{4EI}$$

(2)

iii)



rotation at B

$$EI \theta_B = -\frac{Tl^2}{3} \Rightarrow EI \delta_1 = -\frac{Tl^3}{3}$$

BC as cantilever

$$EI \delta_2 = -\frac{Tl^3}{3}$$

$$\delta = \delta_1 + \delta_2 = -\frac{2Tl^3}{3EI}$$

Compatibility

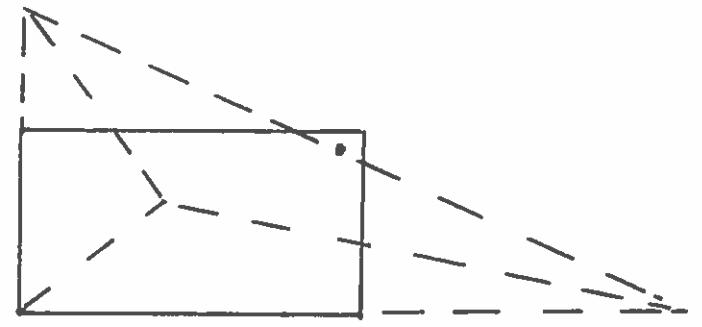
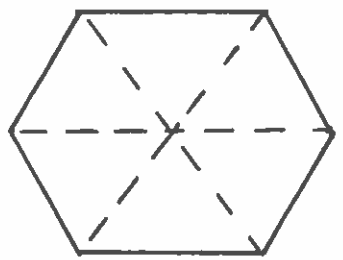
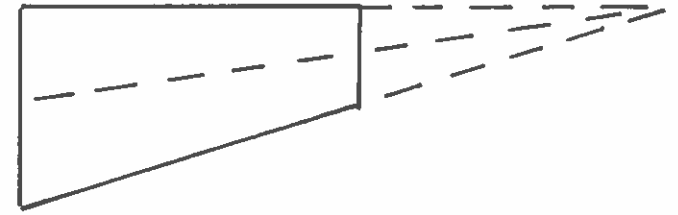
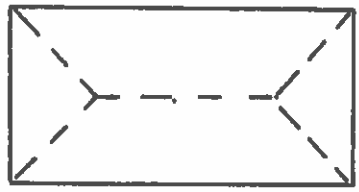
$$\frac{2l^3 T}{3EI} + \frac{T}{EA} \frac{l}{2} = \frac{wl^4}{4EI}$$

$$\Rightarrow T = \frac{3wl^4 EA}{8l^3 EA + 6lEI}$$

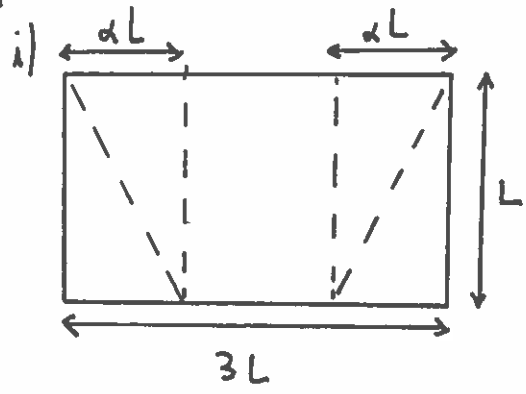
SECTION B

①

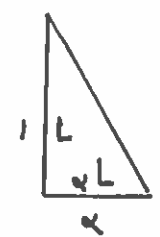
4 a)



b)



Hodograph



Max disp.  $\delta = \alpha L$

Internal work

$$W_i = 2 \sqrt{\alpha^2 L^2 + L^2} m \sqrt{1 + \alpha^2} = 2L(1 + \alpha^2)m$$

External work

$$\begin{aligned} \frac{W_e}{p} &= 4 \frac{L}{2} \alpha L \frac{\delta}{3} + (3L - 2\alpha L) L \frac{\delta}{2} \\ &= \frac{2}{3} \alpha L^2 \delta + 3L^2 \frac{\delta}{2} - \alpha L^2 \delta \\ &= -\frac{1}{3} \alpha L^2 \delta + \frac{3}{2} \alpha L^3 = \frac{1}{6} L^3 \alpha (9 - 2\alpha) \end{aligned}$$

(2)

$$W_c = W_i$$

$$p = \frac{2L(1+\alpha^2)m}{\frac{1}{6}L^3\alpha(9-2\alpha)} = \frac{12(1+\alpha^2)m}{L^2\alpha(9-2\alpha)}$$

$$\text{ii) } \frac{dp}{d\alpha} = 0$$

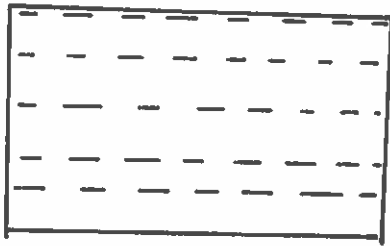
$$\frac{dp}{d\alpha} = \frac{24\alpha m L^2\alpha(9-2\alpha) - 12(1+\alpha^2)m L^2(9-4\alpha)}{\dots} = 0$$

$$= 2\alpha^2(9-2\alpha) - (1+\alpha^2)(9-4\alpha) = 0$$

$$= 18\alpha^2 - 4\alpha^3 - 9 + 4\alpha - 9\alpha^2 + 4\alpha^3 = 0$$

$$= 9\alpha^2 + 4\alpha - 9 = 0 \implies \alpha = 0.802$$

iii)



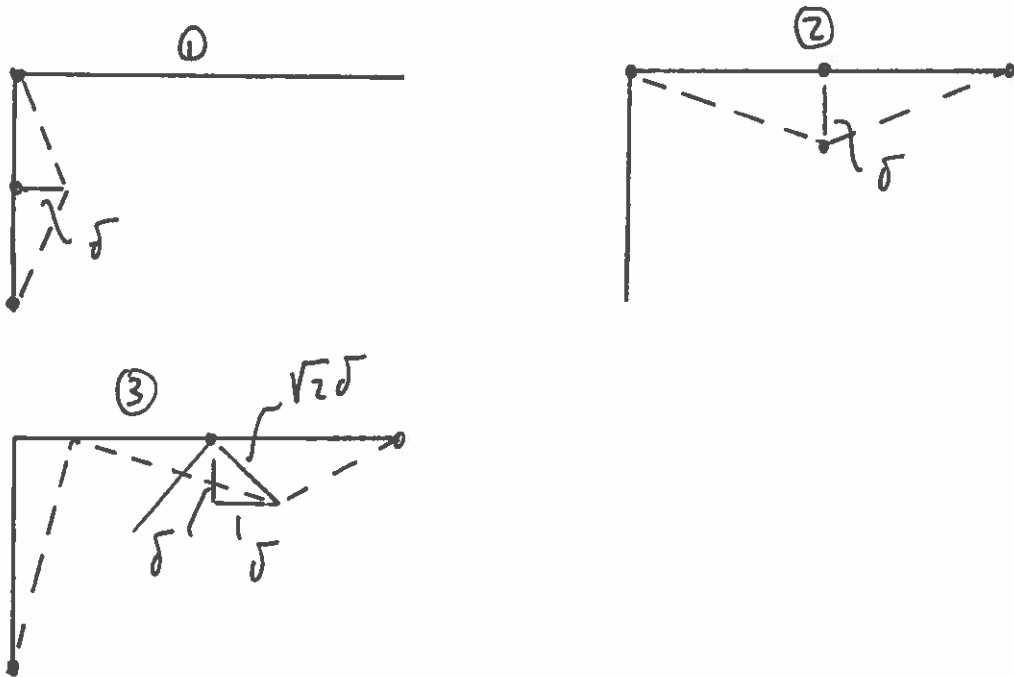
$$m = \frac{pL^2g}{8} \implies p = m \frac{8}{9L^2}$$

Think of strips/beams in horizontal direction; compatibility not important, however equilibrium important.



5) a)

(3)

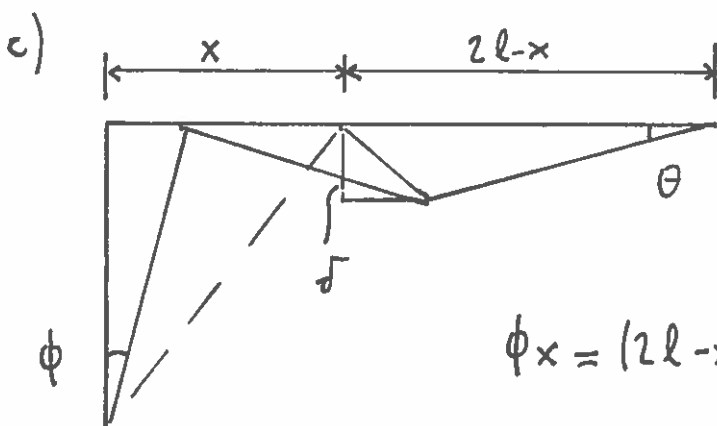


b) Mechanism 1:  $H \cdot \delta = 2 \cdot \left( \frac{2\delta}{l} \right) M_p + 4 \frac{\delta}{l} M_p \Rightarrow H = M_p \frac{8}{l}$

Mechanism 2:  $w \cdot 2l \cdot \frac{\delta}{2} = 4 \frac{\delta}{l} M_p \Rightarrow w = M_p \frac{4}{l^2}$

Mechanism 3:  $H \cdot \frac{\delta}{2} + 2wl \frac{\delta}{2} = M_p \frac{\delta}{l} + M_p \frac{3\delta}{l}$

$$\Rightarrow H + 2wl = \frac{8M_p}{l}$$



$$\phi x = (2l-x)\theta$$

$$\delta = \phi x = (2l-x)\theta$$

$$\Rightarrow \phi = \frac{\delta}{x} \quad \theta = \frac{\delta}{2l-x}$$

(4)

Internal virtual work

$$\begin{aligned}
 W_{\text{int}} &= M_p \phi + M_p \theta + M_p (\theta + \phi) = 2 M_p (\theta + \phi) \\
 &= 2 M_p \delta \left( \frac{1}{x} + \frac{1}{2l-x} \right) = 2 M_p \delta \frac{2l}{x(2l-x)} = \frac{4 M_p \delta l}{x(2l-x)}
 \end{aligned}$$

External virtual work

$$\begin{aligned}
 W_{\text{ext}} &= H \frac{\delta l}{2x} + w \left( x \frac{\delta}{2} + (2l-x) \frac{\delta}{2} \right) \\
 &= H \frac{\delta l}{2x} + \delta \frac{H}{2} = H \delta \frac{l+x}{2x}
 \end{aligned}$$

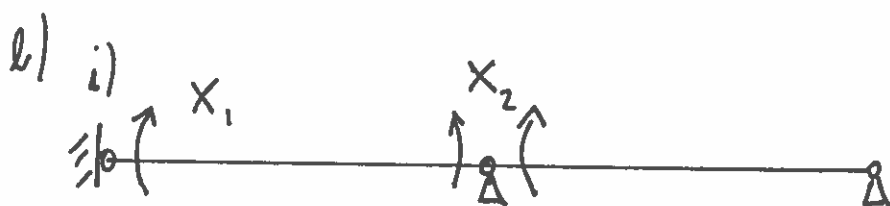
Work equilibrium

$$H = \frac{4 M_p l}{x(2l-x)} \frac{2x}{(l+x)} = \frac{8 M_p l}{2l^2 + lx - x^2}$$

$$\frac{dH}{dx} = - \frac{8 M_p l (l-2x)}{(2l^2 + lx - x^2)^2} = 0 \quad \Rightarrow \quad x = \frac{l}{2}$$

⑤

6 a) equilibrium + material law



from Data Book

$$EI \delta_{11} = \frac{8}{3} X_1$$

$$EI \delta_{12} = \frac{4}{3} X_1$$

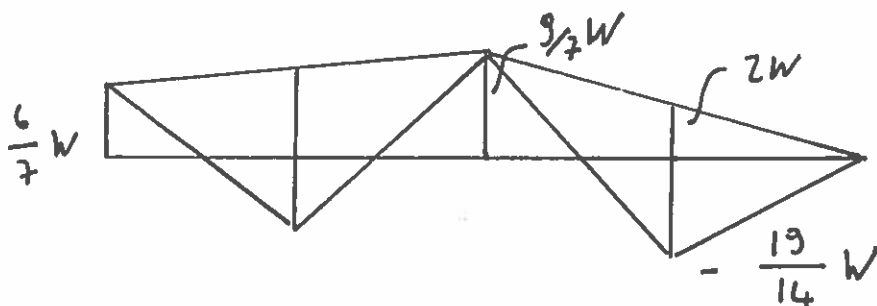
$$EI \delta_{21} = \frac{16}{3} X_2$$

$$\delta_{21} = \delta_{12}$$

$$\delta_{10} = \frac{W64}{16EI}$$

$$\implies EI \delta_{10} = 4W \quad EI \delta_{20} = 8W$$

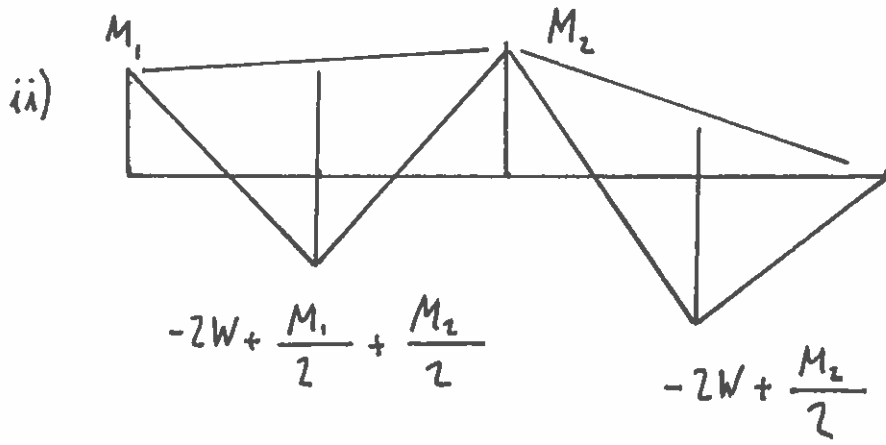
$$\begin{pmatrix} \frac{8}{3} & \frac{4}{3} \\ \frac{4}{3} & \frac{16}{3} \end{pmatrix} \begin{pmatrix} X_1 \\ X_2 \end{pmatrix} = \begin{pmatrix} 4 \\ 8 \end{pmatrix} W \implies \begin{aligned} X_1 &= \frac{6}{7} W \\ X_2 &= \frac{9}{7} W \end{aligned}$$



$$UB \ 356 \times 127 \times 39 \quad z_e = 576 \text{ cm}^3 \quad z_p = 659 \text{ cm}^3$$

$$M_y = 355 \cdot 10^6 \cdot 576 \cdot 10^{-6} = 204.5 \text{ kNm}$$

$$\frac{19}{14} W = 204.5 \implies 150.68 \text{ kN}$$



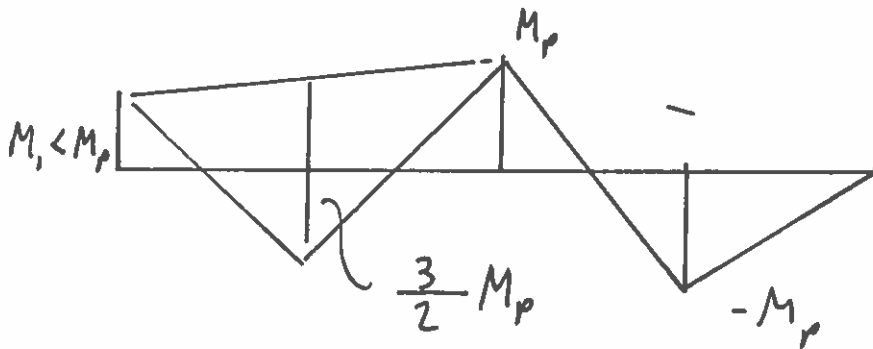
Optimum solution with  
(moments in right span)

$$M_2 = M_p \text{ and } -2W + \frac{M_2}{2} = -M_p$$

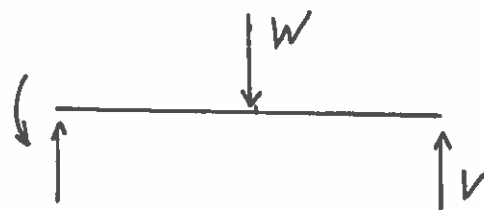
$$\Rightarrow W = \frac{3}{4} M_p$$

$$M_p = 355 \cdot 10^6 \cdot 659 \cdot 10^{-3} = 233.9 \text{ kNm}$$

$$\Rightarrow W = 175.45 \text{ kN}$$



iii)



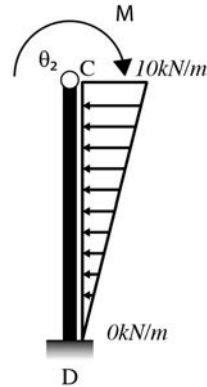
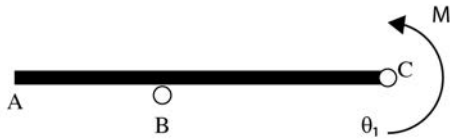
$$M_p - W \cdot 4 + V \cdot 8 = 0 \quad \Rightarrow \quad V = \frac{M_p}{4}$$

$$= 58.5$$

(b)  
(i)

Students must make the frame determinate. Addition of a pin at C is the method given in the question text. There is no horizontal reaction at E therefore:

$$H_D = \frac{10(10)}{2} = 50kN$$



$$\theta_1 = \theta_2$$

$$\theta_1 = \frac{ML}{3EI}$$

$$\theta_2 = \left[ \frac{dv}{dx} @ x = L \right] - \frac{ML}{EI}$$

$$\frac{dv}{dx} = \frac{wL^3}{24} - \frac{wL^3}{4} + \frac{wL^3}{3} = \frac{wL^3}{8EI}$$

$$\theta_2 = \frac{wL^3}{8EI} - \frac{ML}{EI}$$

$$M = 93.75kNm$$

Therefore vertical reactions (and thus maximum axial force in the frame) are:

$$V_E = 93.75 \div 10 = 9.375kN$$

$$V_D = -9.375kN$$