

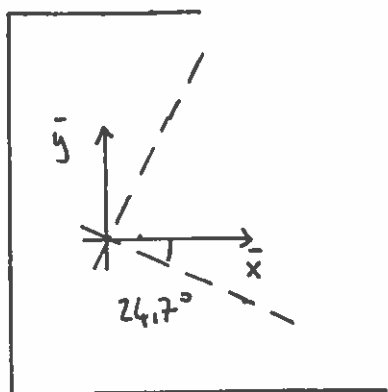
①

Q1 a)

$$i) I = \frac{1}{3} t^3 (2a + 2a + a) = \frac{5}{3} t^3 a$$

$$ii) 5a x_s = 2a \cdot a + a \cdot \frac{a}{2} = \frac{5}{2} a^2 \quad \Rightarrow \quad x_s = \frac{a}{2}$$

$$5a y_s = 2a \cdot a + a \cdot 2a = 4a^2 \quad \Rightarrow \quad y_s = \frac{4}{5} a$$

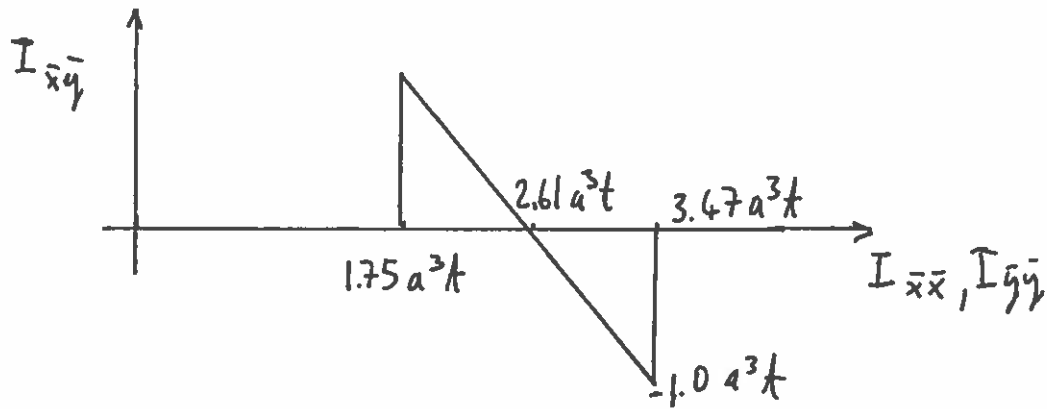


$$I_{\bar{x}\bar{x}} = \frac{8a^3 t}{12} + 2at \left(a - \frac{4}{5}a\right)^2 + 2at \left(\frac{4}{5}a\right)^2 + at \left(2a - \frac{4}{5}a\right)^2 = 3.47 a^3 t$$

$$I_{\bar{y}\bar{y}} = \frac{8a^3 t}{12} + 2at \left(a - \frac{a}{2}\right)^2 + \frac{a^3 t}{12} + at \left(\frac{a}{2} - \frac{a}{2}\right)^2 + 2at \left(\frac{a}{2}\right)^2 = 1.75 a^3 t$$

$$I_{\bar{x}\bar{y}} = 2at \left(-\frac{4}{5}a\right) \left(a - \frac{a}{2}\right) + 2at \left(-\frac{a}{2}\right) \left(-\frac{4}{5}a + a\right) + at \left(2a - \frac{4}{5}a\right) \left(\frac{a}{2} - \frac{a}{2}\right) = -1.0 a^3 t$$

(2)

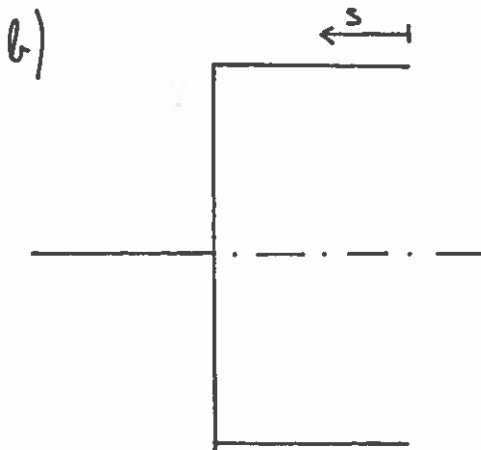


$$R^2 = (3.47 a^3 t - 2.61 a^3 t)^2 + (-1.0 a^3 t)^2 \Rightarrow R = 1.32 a^3 t$$

$$I_{ss} = 2.61 a^3 t - 1.32 a^3 t = 1.29 a^3 t$$

$$I_{tt} = 2.61 a^3 t + 1.32 a^3 t = 3.93 a^3 t$$

$$\tan 2\alpha = \frac{1}{3.47 - 2.61} \Rightarrow \alpha = 24.65^\circ$$



- Shear centre has to be on symmetry axis

Shear flow:

$$q = \frac{F s t a}{I}$$

$$I = \frac{(2a)^3 t}{12} + 2 a t a^2 = \frac{8}{3} a^3 t$$

$$q = F s t a \frac{3}{8 a^3 t} = \frac{3}{8 a^2} F s$$

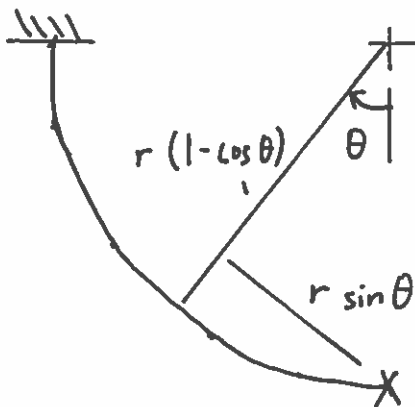
Shear force in top flange  $Q = \int_0^a q ds = \frac{3}{16} F$

$$\frac{3}{16} F \cdot 2a + F x_c = 0 \Rightarrow x_c = -\frac{3}{8} a$$

c) In principal coordinate system

$$q = \frac{F_y \eta t s}{I_{\eta\eta}} + \frac{F_x \xi t s}{I_{\xi\xi}}$$

Q2 a)



$$M = Fr \sin \theta$$

$$T = Fr (1 - \cos \theta)$$

Tip disp. can be computed with principle of virt. disp.

$$\delta = \int_0^{\pi/2} \frac{Fr \sin \theta r \sin \theta}{EI} r d\theta + \int_0^{\pi/2} \frac{Fr (1 - \cos \theta) r (1 - \cos \theta)}{GJ} r d\theta$$

$$EI\delta = Fr^3 \int_0^{\pi/2} \sin^2 \theta d\theta + \frac{Fr^3}{3} \int_0^{\pi/2} (1 - \cos \theta)^2 d\theta$$

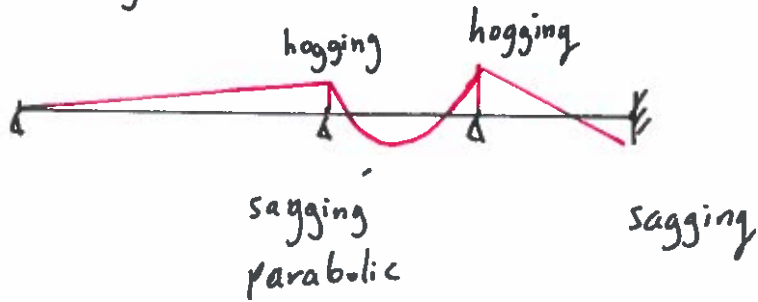
$$= Fr^3 \int_0^{\pi/2} \left( \sin^2 \theta + \frac{1}{3} (1 - 2 \cos \theta + \cos^2 \theta) \right) d\theta$$

$$= Fr^3 \left[ \frac{\pi}{4} + \frac{1}{3} \left( \frac{\pi}{2} - 2 + \frac{\pi}{4} \right) \right] = 0.904 Fr^3$$

bi) Displacements



Bending moments



ii)



Stiffness of beam BA  $M = \frac{3EI}{2l}$

Stiffness matrix of beam BC and BD

$$\begin{bmatrix} \frac{4EI}{l} & \frac{2EI}{l} \\ \frac{2EI}{l} & \frac{4EI}{l} \end{bmatrix}$$

Moments due to loading



$$\begin{bmatrix} \frac{3EI}{2l} + \frac{4EI}{l} & \frac{2EI}{l} \\ \frac{2EI}{l} & \frac{4EI}{l} + \frac{4EI}{l} \end{bmatrix} \begin{bmatrix} r_1 \\ r_2 \end{bmatrix} = \begin{bmatrix} \frac{wl^2}{12} \\ -\frac{wl^2}{12} \end{bmatrix} \quad (5)$$

$$\begin{bmatrix} \frac{11}{2} & 2 \\ 2 & 8 \end{bmatrix} \begin{bmatrix} r_1 \\ r_2 \end{bmatrix} = \frac{wl^3}{EI} \begin{bmatrix} -\frac{1}{12} \\ \frac{1}{12} \end{bmatrix}$$

$$\Rightarrow r_1 = -\frac{1}{48} \frac{wl^3}{EI} \quad r_2 = \frac{1}{64} \frac{wl^3}{EI}$$

$$M_B = -\frac{1}{48} \frac{wl^3}{EI} \cdot \frac{3EI}{2l} = -\frac{1}{32} wl^2$$

(6)

Q3 a)

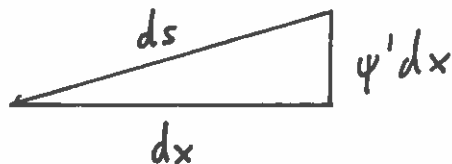
Curvature:  $\kappa = \frac{d^2\psi}{dx^2}$  (for small rotation)

Energy stored in infinitesimal element:  $\frac{1}{2} EI \kappa \cdot \kappa dx$

Total stored energy:

$$U = \int_{-L}^L \frac{1}{2} EI \kappa^2 dx = \frac{1}{2} EI \int_{-L}^L \left( \frac{d^2\psi}{dx^2} \right)^2 dx$$

Axial shortening of infinitesimal element:



$$\psi' = \frac{d\psi}{dx}$$

$$ds = \sqrt{1 + \psi'^2} dx$$

$$\approx \left( 1 + \frac{1}{2} \psi'^2 \right) dx$$

$$ds - dx \approx \frac{\psi'^2}{2} dx$$

Work done by P:

$$P \int_{-L}^L (ds - dx) = P \frac{1}{2} \int_{-L}^L \left( \frac{d\psi}{dx} \right)^2 dx$$

(7)

$$b) \quad \psi_1 = a \left( 1 - \left( \frac{x}{L} \right)^2 \right)$$

$$\frac{d\psi_1}{dx} = -2x \frac{a}{L^2} \quad \frac{d^2\psi_1}{dx^2} = -2 \frac{a}{L^2}$$

$$U = \frac{EI}{2} \int_{-L}^L 4 \frac{a^2}{L^4} dx = 4EI \frac{a^2}{L^3}$$

$$V = \frac{P}{2} \int_{-L}^L 4x^2 \frac{a^2}{L^4} dx = \frac{4}{3} \frac{P}{L} a^2$$

For buckling load  $U=V$ :

$$4EI \frac{a^2}{L^3} = \frac{4}{3} \frac{P}{L} a^2 \quad \implies P = 3 \frac{EI}{L^2}$$

c)

$$\psi_2 = \frac{a}{L^2} (L^2 - x^2) + \frac{b}{L^4} (L^4 - x^4)$$

$$\frac{d\psi_2}{dx} = -2x \frac{a}{L^2} - 4x^3 \frac{b}{L^4}$$

$$\frac{d^2\psi_2}{dx^2} = -2 \frac{a}{L^2} - 12x^2 \frac{b}{L^4}$$

$$\left( \frac{d^2\psi_2}{dx^2} \right)^2 = \begin{pmatrix} \frac{a}{L^2} & \frac{b}{L^4} \end{pmatrix} \begin{pmatrix} 4 & 24x^2 \\ 24x^2 & 144x^4 \end{pmatrix} \begin{pmatrix} \frac{a}{L^2} \\ \frac{b}{L^4} \end{pmatrix}$$

(8)

$$\int_{-L}^L \left( \frac{d^2 \psi_c}{dx^2} \right)^2 dx = \begin{pmatrix} \frac{a}{L^2} & \frac{b}{L^4} \end{pmatrix} \begin{pmatrix} 8L & 16L^3 \\ 16L^3 & \frac{288}{5} L^5 \end{pmatrix} \begin{pmatrix} \frac{a}{L^2} \\ \frac{b}{L^4} \end{pmatrix}$$

$$\left( \frac{d\psi_c}{dx} \right)^2 = \begin{pmatrix} \frac{a}{L^2} & \frac{b}{L^4} \end{pmatrix} \begin{pmatrix} 4x^2 & 8x^4 \\ 8x^4 & 16x^6 \end{pmatrix} \begin{pmatrix} \frac{a}{L^2} \\ \frac{b}{L^4} \end{pmatrix}$$

$$\int_{-L}^L \left( \frac{d\psi_c}{dx} \right)^2 dx = \begin{pmatrix} \frac{a}{L^2} & \frac{b}{L^4} \end{pmatrix} \begin{pmatrix} \frac{8}{3} L^3 & \frac{16}{5} L^5 \\ \frac{16}{5} L^5 & \frac{32}{7} L^7 \end{pmatrix} \begin{pmatrix} \frac{a}{L^2} \\ \frac{b}{L^4} \end{pmatrix}$$

$$U - V = \frac{EI}{L^3} \begin{pmatrix} a & b \end{pmatrix} \begin{pmatrix} 4 - \frac{4}{3} P' & 8 - \frac{8}{5} P' \\ 8 - \frac{8}{5} P' & \frac{144}{5} - \frac{16}{7} P' \end{pmatrix} \begin{pmatrix} a \\ b \end{pmatrix}$$

$$\text{with } P' = \frac{PL^2}{EI}$$

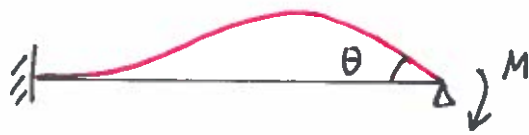
Matrix must be singular at buckling.



Q 4

(9)

a)



From Data Book:  $\frac{M}{\theta} = \frac{2EI}{L}$

For both sides stiffness is doubled:  $\frac{M}{\theta} = 4 \frac{EI}{L}$

b) Consider AB



$$\begin{pmatrix} M_A \\ M_B \end{pmatrix} = \frac{EI}{L} \begin{pmatrix} s & sc \\ sc & s \end{pmatrix} \begin{pmatrix} \theta_A \\ \theta_B \end{pmatrix}$$

$$\theta_B = 0$$

Add beam stiffness to A:  $M_A = (s+4) \theta_A \frac{EI}{L}$

Buckling occurs when  $s+4 = 0$

$P/P_E$	$s$	$s+4$
2.8	-3.4449	0.5551
3.0	-5.0320	-1.0320

$$\Rightarrow \frac{P}{P_E} = 2.82 \quad (\text{using interpolation})$$

$$P = 2.82 \frac{\pi^2 EI}{L^2} = 27.8 \frac{EI}{L^2}$$

c)

(10)

$$\begin{pmatrix} M_A \\ M_B \end{pmatrix} = \frac{EI}{L} \begin{pmatrix} s+4 & sc \\ sc & 2s+4 \end{pmatrix} \begin{pmatrix} \theta_A \\ \theta_B \end{pmatrix}$$

Matrix needs to be singular

$$(s+4)(2s+4) - (sc)^2 = 0$$

By trying different  $s$  and  $c$  values from the table and interpolation:

$$\frac{P}{P_E} = 2.18$$

$$P = 2.18 \frac{\pi^2 EI}{L^2} = 21.5 \frac{EI}{L^2}$$

Mode of buckling (using  $P/P_E = 2.2$ )

$$\frac{EI}{L} \begin{pmatrix} 3.4806 & 3.5249 \\ 3.5249 & 4.1552 \end{pmatrix} \begin{pmatrix} \theta_A \\ \theta_B \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \end{pmatrix}$$

Top row:  $\frac{\theta_B}{\theta_A} = -0.989$

Bottom row:  $\frac{\theta_B}{\theta_A} = -0.848$

Take average  $\frac{\theta_B}{\theta_A} \approx -0.92$