

EGT2  
ENGINEERING TRIPOS PART IIA

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Monday 23 April 2018 2 to 3.40

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**Module 3D4**

**STRUCTURAL ANALYSIS AND STABILITY**

*Answer not more than **three** questions.*

*All questions carry the same number of marks.*

*The **approximate** percentage of marks allocated to each part of a question is indicated in the right margin.*

*Write your candidate number **not** your name on the cover sheet.*

**STATIONERY REQUIREMENTS**

Single-sided script paper

Graph paper

**SPECIAL REQUIREMENTS TO BE SUPPLIED FOR THIS EXAM**

CUED approved calculator allowed

Attachment: Data Sheet for Question 2: Stiffness Matrices (1 page)

Engineering Data Book

**10 minutes reading time is allowed for this paper at the start of the exam.**

**You may not start to read the questions printed on the subsequent pages of this question paper until instructed to do so.**

1 (a) Figure 1(a) shows a thin-walled unsymmetric channel-like cross-section with constant thickness  $t$ .

(i) Compute the St. Venant torsion constant of the cross-section. [10%]

(ii) Compute directions of the principal axes and the principal second moments of area of the cross-section. [40%]

(b) Figure 1(b) shows a thin-walled channel-like cross section with a single symmetry axis and a constant thickness  $t$ . Compute the shear centre of the cross-section. [35%]

(c) Describe how you would compute the shear centre of the cross-section shown in Fig. 1(a). No numerical computations are necessary. [15%]

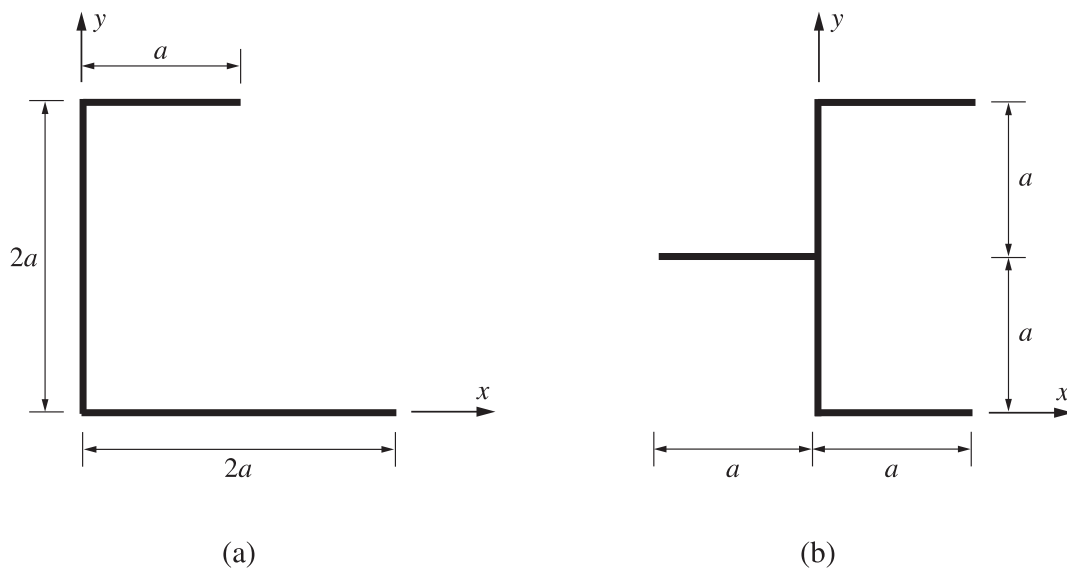


Fig. 1

2 (a) Figure 2(a) shows a circular arc cantilever with radius  $r$  which lies in the  $x-y$  plane. The flexural stiffness of the cantilever is  $EI$  and its torsional stiffness is  $GJ = 3EI$ . It is loaded at its free end by a force  $F$  acting in the negative  $z$ -direction. Compute the tip displacement at the cantilever's free end. [35%]

(b) Figure 2(b) shows the side view of a continuous beam over three spans with uniform flexural rigidity  $EI$ . The beam is simply supported at its left end, clamped at its right end and continuous over the two mid-span supports. The centre span carries a uniformly distributed load  $w$ .

(i) Sketch the bending moment diagram over the entire beam length, marking its salient features. No numerical computations are necessary. [20%]

(ii) Compute the hogging moment over support B using the displacement method. [45%]

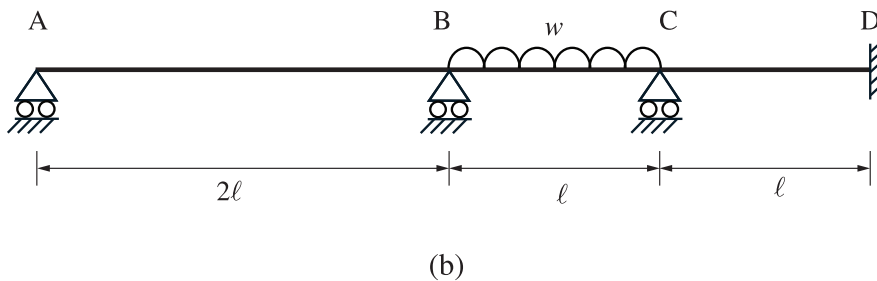
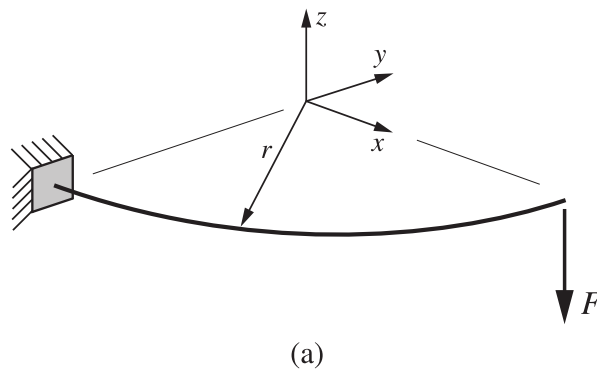


Fig. 2

3 Figure 3(a) shows a pin-ended strut with bending stiffness  $EI$  carrying a compressive load  $P$ . Consider the deformation of the strut shown in Fig. 3(b), where the strut takes up a shape  $y = \psi(x)$ .

(a) Show that the bending energy  $U$  stored in the strut is given by

$$U = \frac{1}{2}EI \int_{-L}^L \left( \frac{d^2\psi}{dx^2} \right)^2 dx$$

and that the work  $V$  done by the load  $P$  is given by

$$V = \frac{1}{2}P \int_{-L}^L \left( \frac{d\psi}{dx} \right)^2 dx.$$

[30%]

(b) Estimate the buckling load of the strut using a mode shape

$$\psi_1 = a \left( 1 - \left( \frac{x}{L} \right)^2 \right).$$

[20%]

(c) The buckling load of the strut will be estimated using a mode shape

$$\psi_2 = a \left( 1 - \left( \frac{x}{L} \right)^2 \right) + b \left( 1 - \left( \frac{x}{L} \right)^4 \right).$$

The total potential energy may be expressed as

$$U - V = \frac{1}{2} \begin{bmatrix} a & b \end{bmatrix} \begin{bmatrix} \mathbf{K} \end{bmatrix} \begin{bmatrix} a \\ b \end{bmatrix}.$$

Determine the matrix  $\mathbf{K}$  and without further calculation explain how this may be used to compute the buckling load. [50%]



Fig. 3

4 Figure 4 shows three frame structures that are all braced against out-of-plane displacements. Each member has flexural rigidity  $EI$  for bending deformations in the plane of the frame.

(a) The central point A of the beam shown in Fig. 4(a) has a support that prevents displacement but not rotation. Show that the stiffness of the structure against rotation at A is  $4EI/L$ . [10%]

(b) A vertical load  $F$  is applied at A to the frame shown in Fig. 4(b). Using the  $s$  &  $c$  functions given in Table 1, where the stiffness factor  $s$  and carry-over factor  $c$  are given for a column with Euler buckling load  $P_E$  carrying a compressive load  $P$ , estimate the critical load to cause buckling. [40%]

(c) A vertical load  $F$  is applied at A to the frame shown in Fig. 4(c). Again using  $s$  &  $c$  functions, estimate the critical load to cause buckling, and describe the mode of buckling. [50%]

$P/P_E$	$s$	$c$	$P/P_E$	$s$	$c$
0.0	4.0000	0.5000	1.6	1.2240	2.4348
0.2	3.7297	0.5550	1.8	0.7170	4.4969
0.4	3.4439	0.6242	2.0	0.1428	24.6841
0.6	3.1403	0.7136	2.2	-0.5194	-7.5107
0.8	2.8159	0.8330	2.4	-1.3006	-3.3703
1.0	2.4674	1.0000	2.6	-2.2490	-2.2312
1.2	2.0901	1.2487	2.8	-3.4449	-1.7081
1.4	1.6782	1.6557	3.0	-5.0320	-1.4157

Table 1

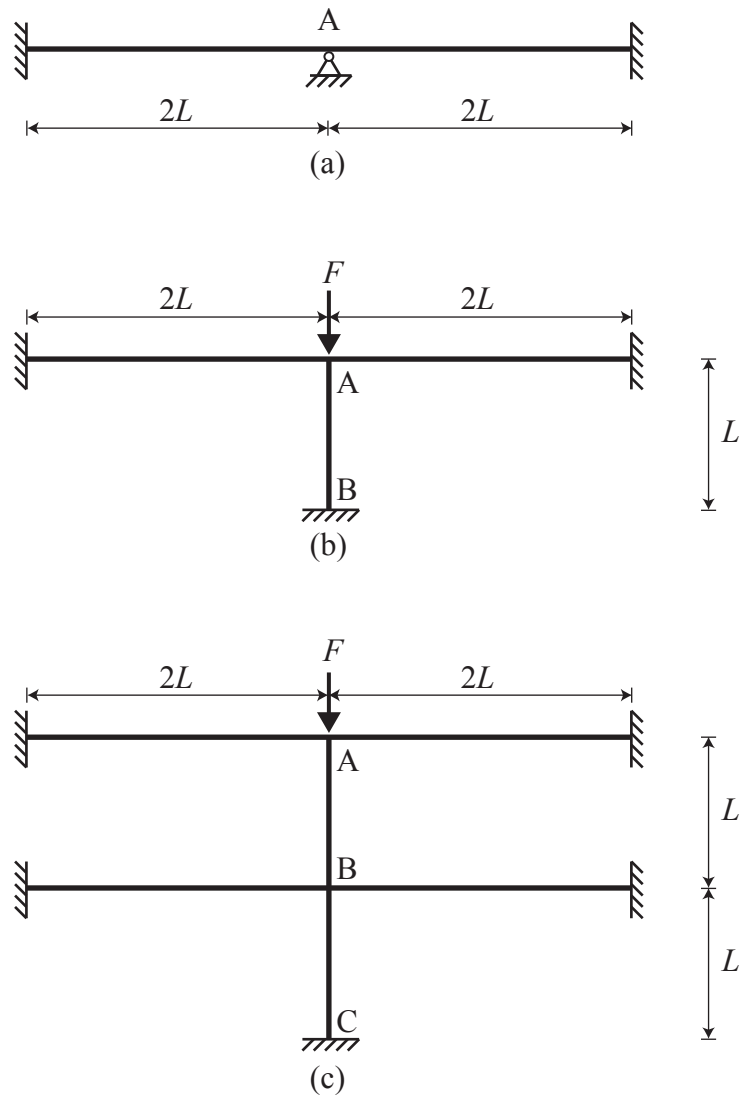
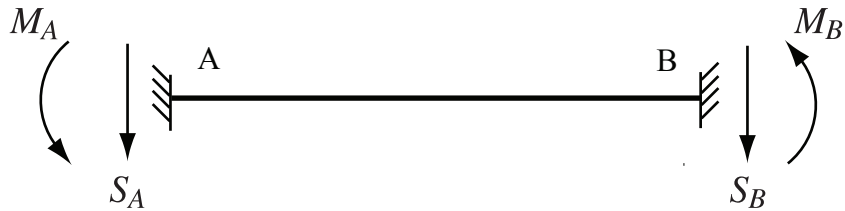


Fig. 4

**END OF PAPER**

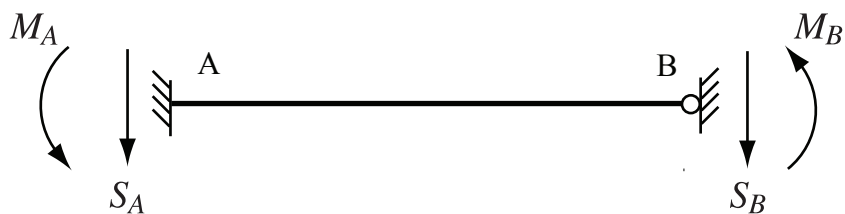
**Data Sheet for Question 2: Stiffness Matrices.**

**Beam type I**



$$\begin{bmatrix} S_A \\ M_A \\ S_B \\ M_B \end{bmatrix} = \begin{bmatrix} \frac{12EI}{L^3} & -\frac{6EI}{L^2} & -\frac{12EI}{L^3} & -\frac{6EI}{L^2} \\ -\frac{6EI}{L^2} & \frac{4EI}{L} & \frac{6EI}{L^2} & \frac{2EI}{L} \\ -\frac{12EI}{L^3} & \frac{6EI}{L^2} & \frac{12EI}{L^3} & \frac{6EI}{L^2} \\ -\frac{6EI}{L^2} & \frac{2EI}{L} & \frac{6EI}{L^2} & \frac{4EI}{L} \end{bmatrix} \begin{bmatrix} w_A \\ \phi_A \\ w_B \\ \phi_B \end{bmatrix}$$

**Beam type II**



$$\begin{bmatrix} S_A \\ M_A \\ S_B \\ M_B \end{bmatrix} = \begin{bmatrix} \frac{3EI}{L^3} & -\frac{3EI}{L^2} & -\frac{3EI}{L^3} & 0 \\ -\frac{3EI}{L^2} & \frac{3EI}{L} & \frac{3EI}{L^2} & 0 \\ -\frac{3EI}{L^3} & \frac{3EI}{L^2} & \frac{3EI}{L^3} & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} w_A \\ \phi_A \\ w_B \\ \phi_B \end{bmatrix}$$