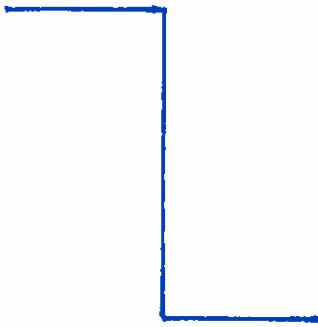


1a)



$$i) J = \frac{2}{3} b t^3 + \frac{1}{3} b t^3 = \frac{5}{3} b t^3$$

$$ii) \Gamma = \frac{(3b)^2}{4} I_{yy}$$

$$I_{yy} = 2 \left[\frac{b^3 t}{12} + b t \left(\frac{b}{2} \right)^2 \right] = \frac{1}{6} b^3 t + \frac{1}{2} b^3 t = \frac{2}{3} b^3 t$$

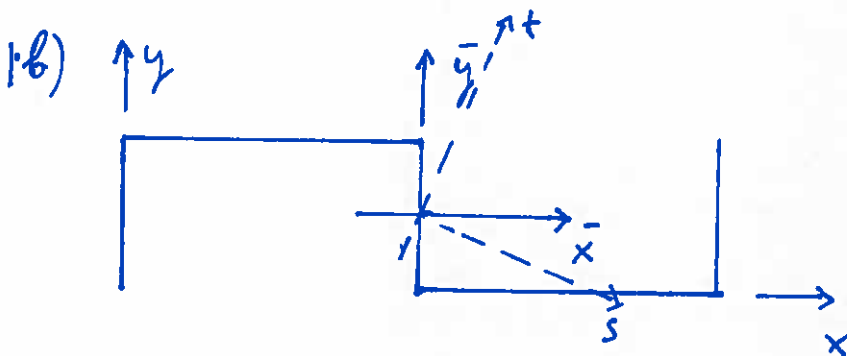
$$\Rightarrow \Gamma = \frac{9}{4} b^2 \frac{2}{3} b^3 t = \frac{3}{2} b^5 t$$

$$\lambda^2 = \frac{E}{G} \frac{3}{2} b^5 t \frac{3}{5} b^3 t \Rightarrow \lambda = \frac{3b^2}{t} \sqrt{\frac{E}{10G}}$$

$$T = GJ \phi' \Rightarrow \phi' = \frac{T}{GJ}$$

$$\phi = (L - \lambda) \frac{T}{GJ} = \left(L - \frac{3b^2}{t} \sqrt{\frac{E}{10G}} \right) \frac{3Tb}{5bt^3G}$$

$$= 0,6 \frac{T}{Gbt^3} \left(L - 0,949 \frac{b^2}{t} \sqrt{\frac{E}{G}} \right)$$



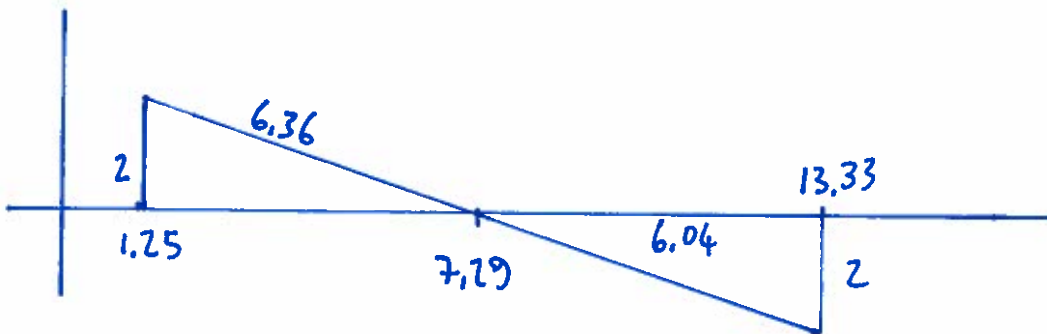
Centroid: $x_s = 2b$ $y_s = \frac{b}{2}$

(2)

$$I_{xx} = 3 \frac{b^3 t}{12} + 2 \left(\frac{b}{2} \right)^2 2bt = \frac{5}{4} b^3 t$$

$$I_{yy} = 2 \left(bt \cdot (2b)^2 \right) + 2 \left(\frac{8b^3}{12} t + 2bt \cdot b^2 \right) = \frac{40}{3} b^3 t$$

$$I_{xy} = 2bt(-b) \frac{b}{2} + 2bt(b) \left(-\frac{b}{2} \right) = -2b^3 t$$

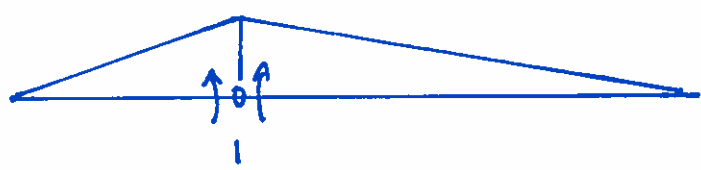


$$I_{ss} = 13.65 b^3 t$$

$$I_{tt} = 0.928 b^3 t$$

$$\tan(2\theta) = \frac{2}{6.04} \implies \theta = 9.16^\circ$$

2a)



$$EI \delta_{11} = \frac{1}{3}L + \frac{2}{3}L = L$$



$$EI \delta_{10} = \frac{2L}{3} L^2 w = \frac{L^3 w}{3}$$

$$w \frac{4L^2}{8} = \frac{L^2}{2} w$$

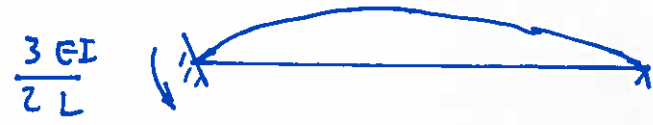
$$X \delta_{11} = \delta_{10}$$

$$\Rightarrow X = \frac{\delta_{10}}{\delta_{11}} = \frac{L^3 w}{3L} = \frac{L^2 w}{3}$$

2b)



$$\frac{w(2L)^2}{8} = \frac{wL^2}{2}$$



$$\left(\frac{3}{2} \frac{EI}{L} + \frac{3EI}{L} \right) r = \frac{wL^2}{2} \Rightarrow EI r = \frac{wL^3}{9}$$

$$\Rightarrow M_B = \frac{3EI}{L} \frac{wL^3}{9EI} = \frac{L^2 w}{3}$$

2c)



$$-EIw'' = -R_A z - R_B \{z-L\} + w \frac{\{z-L\}^2}{2}$$

$$-EIw' = -R_A \frac{z^2}{2} - \frac{R_B}{2} \left\{ \frac{z-L}{2} \right\}^2 + \frac{w}{6} \{z-L\}^3 + C_1$$

$$-EIw = -R_A \frac{z^3}{6} - \frac{R_B}{6} \left\{ \frac{z-L}{2} \right\}^3 + \frac{w}{24} \{z-L\}^4 + C_1 x + C_2$$

Five unknowns; need five equations

$w=0$ at $z=0$, $z=L$ and $z=3L$

$R_A + R_B + R_C = 2Lw$ vertical equilibrium

$3R_C L + R_B L = 2wL(2L) = w4L^2$ moment equilibrium about $z=0$

$$w(0) = 0 \Rightarrow C_2 = 0$$

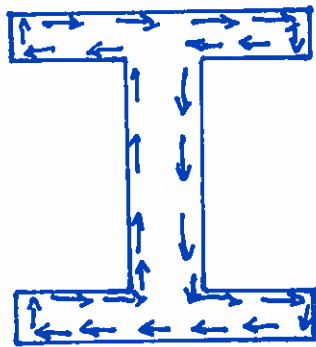
$$w(L) = 0 \quad -R_A \frac{L^3}{6} + C_1 L = 0 \Rightarrow C_1 = R_A \frac{L^2}{6}$$

$$w(3L) = 0 \quad -R_A \frac{9L^3}{2} - R_B \frac{L^3}{6} + w \frac{2}{3} L^4 + R_A \frac{L^3}{2} = 0$$

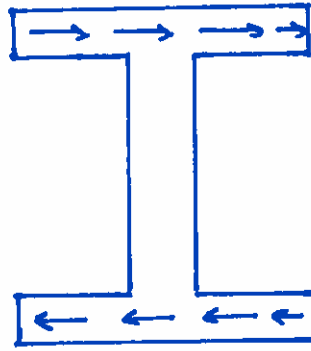
$$\begin{pmatrix} 1 & 1 & 1 \\ 0 & 1/3 & 1 \\ 4 & 1/6 & 0 \end{pmatrix} \begin{pmatrix} R_A \\ R_B \\ R_C \end{pmatrix} = \begin{pmatrix} 2Lw \\ \frac{4wL}{3} \\ \frac{2wL}{3} \end{pmatrix}$$

$$2d) \quad -EIw' = -R_A \frac{z^2}{2} - R_B \frac{1}{2} \left\{ \frac{z-L}{2} \right\}^2 + 1 \cdot \left\{ z - \frac{L}{2} \right\}^0 + C_1$$

3a)



St. Venant torsion



Restraint warping torsion

$$3b) \quad M_{ct} = \frac{\pi}{L} \sqrt{GJ EI_{min}} \left(1 + \frac{\pi^2 E \Gamma}{L^2 GJ} \right)^{\frac{1}{2}}$$

$$L = 6 \text{ m}$$

$$E = 210 \cdot 10^9 \text{ N/m}^2 \quad G = E/2.6 \quad (\nu = 0.3)$$

$$I_{min} = 2692 \cdot 10^{-8} \text{ m}^4$$

$$J = 101 \cdot 10^{-8} \text{ m}^4$$

$$D_f = 536.7 - 17.4 = 519.3 \text{ mm} = 0.5193 \text{ m}$$

$$\Gamma = I_{min} \frac{D_f^2}{4} = \frac{2692 \cdot 10^{-8}}{4} (0.5193)^2 = 181.5 \text{ m}^6 \cdot 10^{-8}$$

$$\frac{1 + \pi^2 E \Gamma}{L^2 GJ} = 1 + \frac{\pi^2}{36} \cdot 2.6 \frac{181.5}{101} = 2.28$$

$$M_{basic} = \frac{\pi}{L} \sqrt{GJ EI_{min}} = \frac{\pi}{L} E \sqrt{\frac{I_{min} J}{2.6}} =$$

$$= \frac{\pi}{6} 210 \cdot 10^9 \sqrt{\frac{101 \cdot 2692}{2.6} \cdot 10^{-8}} = 355.6 \text{ kNm}$$

$$M_{LT} = 355.6 \sqrt{2.28} = \underline{\underline{536.9 \text{ kNm}}}$$

3c)



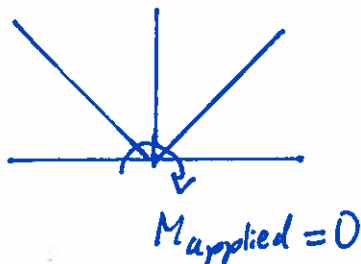
$$\begin{bmatrix} M_A \\ M_B \end{bmatrix} = \frac{EI}{L} \begin{bmatrix} s & sc \\ sc & s \end{bmatrix} \begin{bmatrix} \theta_A \\ \theta_B \end{bmatrix}$$

$$M_B = 0 \Rightarrow sc \theta_A + s \theta_B = 0 \Rightarrow \theta_B = -c \theta_A$$

$$\begin{aligned} M_A &= \frac{EI}{L} (s \theta_A + sc \theta_B) = \frac{EI}{L} (s \theta_A - sc^2 \theta_A) \\ &= \frac{EI}{L} s (1 - c^2) \theta_A \end{aligned}$$

$$\text{if } P=0 \quad s=4 \quad c=\frac{1}{2} \Rightarrow s(1-c^2) = 4(1-\frac{1}{4}) = 3$$

For the framework



$$M_{\text{applied}} = \frac{EI}{L} (2s(1-c^2) + 9) = 0$$

Buckling condition is $2s(1-c^2) + 9 = 0$

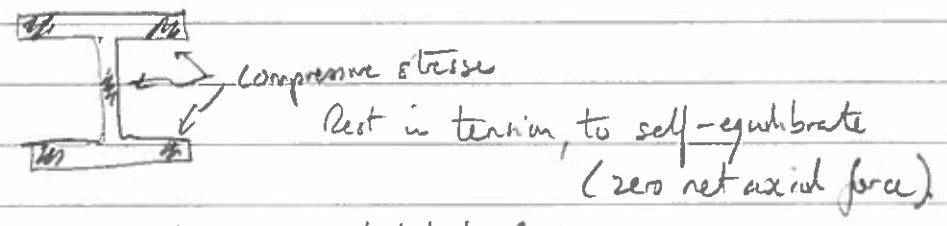
$$s(1-c^2) = -4.5$$

P/P_E	s	c	$s(1-c^2)$
1.6	1.224	2.4348	-6.632
1.4	1.6782	1.6557	-2.922

$$\Rightarrow P/P_E = 1.502$$

$$\begin{aligned} P &= \frac{\pi^2 EI}{L^2} \cdot 1.502 \\ &= 14.82 \frac{EI}{L^2} \end{aligned}$$

3D4 Q4. a)
Residual stresses.

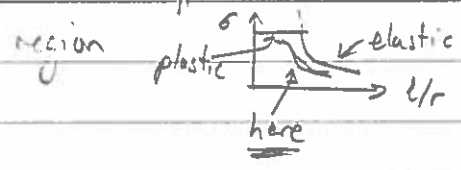


(Due to exposed areas solidifying first, and then as rest cools + contracts, it pulls these into compression).

Can affect buckling behaviour, particularly the flange outstands.

→ can precipitate local buckling of flange outstands due to pre-existing compression there

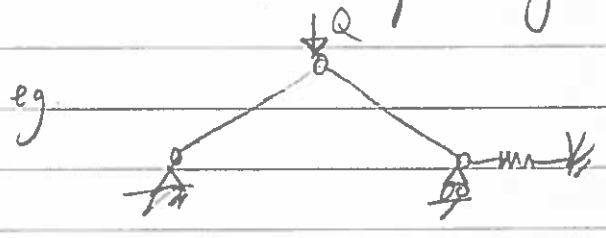
→ can affect overall buckling, particularly in elasto-plastic region, because as the member starts to



buckle, it encounters plastic behaviour sooner than it would if no residual stresses.

- b). - clear demarcation between "axial" and "lateral" loads
- no "lateral" loads
- axial end shortening \propto (lateral deflection)²
- no lateral deflection when axial load is zero

These do not hold for snap-through buckling



(c) Rayleigh-Ritz.

TOT. POT. ENERGY $\Pi =$ strain energy - ext. W.D.

$$\Pi = \frac{1}{2} \int EI (w'')^2 dx - \frac{P}{2} \int (w')^2 dx$$

$$w = A \phi(x)$$

$$\Pi = \frac{1}{2} A^2 \int EI (\phi'')^2 dx - \frac{P}{2} A^2 \int (\phi')^2 dx$$

$$= \frac{1}{2} k A^2 \text{ with } k = \int EI (\phi'')^2 dx - P \int (\phi')^2 dx$$

so buckles when stiffness $k \rightarrow 0$

$$\Rightarrow P = \frac{\int EI (\phi'')^2 dx}{\int (\phi')^2 dx}$$

Let $\phi = x(L-x) = Lx - x^2$

$$\phi' = L - 2x$$

$$\phi'' = -2$$

$$\int EI (\phi'')^2 dx = EI (4) L$$

$$\int (\phi')^2 dx = \int_0^L (L-2x)^2 dx$$

$$= \int_0^L L^2 - 4xL + 4x^2 dx$$

$$= \left[L^2 x - \frac{4x^2 L}{2} + \frac{4x^3}{3} \right]_0^L = L^3 - 2L^3 + \frac{4L^3}{3} = \frac{L^3}{3}$$

$$P_{cr} = \frac{4EI L}{L^3/3} = \frac{12EI}{L^2}$$

This is greater than Euler load $\left(\frac{\pi^2 EI}{L^2} \right)$ as $12 > \pi^2 \approx 10$.

It is higher because Rayleigh-Ritz always overestimates (or is correct). It is correct if ϕ is an eigenvector. In this case, eigenvector is half-sine wave (not quadratic). Total potential energy goes flat in eigendirection first, then other directions later (hence over estimate).

3D4 2016

Q4 a)

Perry-Robertson deals with inelasticity by avoiding it altogether.

As soon as any part of the cross-section yields, that is taken as "failure", and determines the "design" load.

This avoids all the philosophical complications associated with the path-dependent nature of inelastic behaviour, as exemplified by say Shanley's analysis which resolved the column paradox.