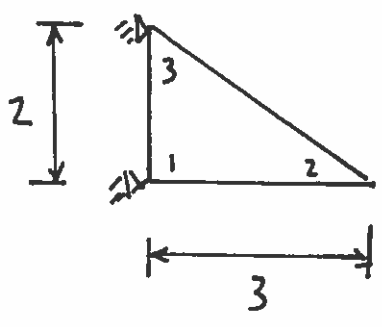


1a)



$$N_2 = \frac{x}{3}$$

$$B = \begin{bmatrix} \frac{1}{3} & 0 \\ 0 & 0 \\ 0 & \frac{1}{3} \end{bmatrix}$$

$$D = \frac{200}{1-0,3^2} \begin{bmatrix} 1 & 0,3 & 0 \\ 0,3 & 1 & 0 \\ 0 & 0 & 0,35 \end{bmatrix} = 219,8 \begin{bmatrix} 1 & 0,3 & 0 \\ 0,3 & 1 & 0 \\ 0 & 0 & 0,35 \end{bmatrix}$$

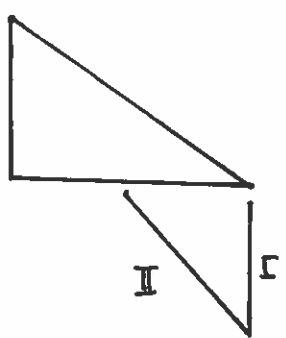
$$K = 219,8 \cdot 3 \begin{bmatrix} \frac{1}{3} & 0 & 0 \\ 0 & 0 & \frac{1}{3} \end{bmatrix} \begin{bmatrix} 1 & 0,3 & 0 \\ 0,3 & 1 & 0 \\ 0 & 0 & 0,35 \end{bmatrix} \begin{bmatrix} \frac{1}{3} & 0 \\ 0 & 0 \\ 0 & \frac{1}{3} \end{bmatrix}$$

$$\begin{bmatrix} \frac{1}{3} & \frac{0,3}{3} & 0 \\ 0 & 0 & \frac{0,35}{3} \end{bmatrix}$$

$$\begin{bmatrix} \frac{1}{9} & 0 \\ 0 & \frac{0,35}{9} \end{bmatrix}$$

$$K = \begin{bmatrix} 73.27 & 0 \\ 0 & 25.64 \end{bmatrix}$$

b)



$$K^I = \frac{EA}{1.5} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = \begin{bmatrix} 66.7 & -66.7 \\ -66.7 & 66.7 \end{bmatrix}$$

$$K^{II} = \frac{EA}{2.12} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = \begin{bmatrix} 47.17 & -47.17 \\ -47.17 & 47.17 \end{bmatrix}$$

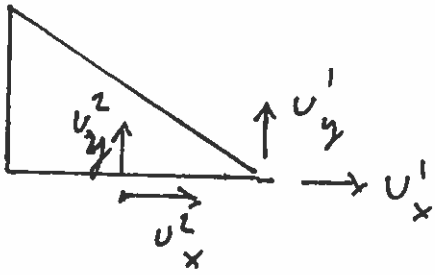
Consider bar II

$$\begin{bmatrix} u_{\tilde{x}} \\ u_{\tilde{y}} \end{bmatrix} = \begin{bmatrix} \sqrt{2}/2 & \sqrt{2}/2 \\ -\sqrt{2}/2 & \sqrt{2}/2 \end{bmatrix} \begin{bmatrix} u_x \\ u_y \end{bmatrix}$$

$$\begin{bmatrix} \sqrt{2}/2 & -\sqrt{2}/2 \\ \sqrt{2}/2 & \sqrt{2}/2 \end{bmatrix} \begin{bmatrix} 0 & 0 \\ 0 & 47.17 \end{bmatrix} \begin{bmatrix} \sqrt{2}/2 & \sqrt{2}/2 \\ -\sqrt{2}/2 & \sqrt{2}/2 \end{bmatrix}$$

$$\begin{bmatrix} 0 & -33.35 \\ 0 & 33.35 \end{bmatrix}$$

$$\begin{bmatrix} 23.59 & -23.59 \\ -23.59 & 23.59 \end{bmatrix}$$



Global stiffness

$$\begin{pmatrix} 73.27 & 0 & 0 & 0 \\ 0 & 25.64 + 66.7 & 0 & 0 \\ 0 & 0 & 23.59 & -23.59 \\ 0 & 0 & -23.59 & 23.59 \end{pmatrix} \begin{pmatrix} u'_x \\ u'_y \\ u^z_x \\ u^z_y \end{pmatrix} = \begin{pmatrix} 0 \\ f_y \\ 0 \\ 0 \end{pmatrix}$$

Note that

$$u^z_x = \frac{1}{2} u'_x$$

$$u^z_y = \frac{1}{2} u'_y$$

leading to \Rightarrow

$$\begin{pmatrix} u'_x \\ u'_y \\ u^z_x \\ u^z_y \end{pmatrix} = \begin{pmatrix} 1 & 0 \\ 0 & 1 \\ \frac{1}{2} & 0 \\ 0 & \frac{1}{2} \end{pmatrix} \begin{pmatrix} u'_x \\ u'_y \end{pmatrix}$$

$$\begin{pmatrix} 1 & 0 & 0.5 & 0 \\ 0 & 1 & 0 & 0.5 \end{pmatrix} \begin{pmatrix} 73.27 & 0 & 0 & 0 \\ 0 & 92.34 & 0 & 0 \\ 0 & 0 & 23.59 & -23.59 \\ 0 & 0 & -23.59 & 23.59 \end{pmatrix} \begin{pmatrix} u'_x \\ u'_y \\ u^z_x \\ u^z_y \end{pmatrix} = \begin{pmatrix} 0 \\ f_y \\ 0 \\ 0 \end{pmatrix}$$

$$\begin{pmatrix} 73.27 & 0 & 11.80 & -11.80 \\ 0 & 92.34 & -11.80 & 11.80 \end{pmatrix}$$

$$\begin{pmatrix} 79.17 & -5.80 \\ -5.80 & 98.24 \end{pmatrix} \begin{pmatrix} u'_x \\ u'_y \end{pmatrix} = \begin{pmatrix} 0 \\ f_y \end{pmatrix}$$

$$\Rightarrow u'_x = 7.49 \cdot 10^{-4} \quad u'_y = 1.02 \cdot 10^{-2}$$

2) a) Too few integration points lead to zero energy modes. Need to increase number of integration points.

$$b) \int (\nu^h - f) w^h d\Omega = 0$$

$$\int \left(\sum_I N_I \nu_I - f \right) \sum_J N_J w_J d\Omega = 0$$

$$\sum_J w_J \underbrace{\left(\sum_I N_I N_J \nu_I - f N_J \right)}_{= 0 \text{ for arbitrary } w_I} = 0$$

\Rightarrow

$$\sum_J \sum_I \int N_I N_J \nu_I = \sum_J \int f N_J$$

$$M_{IJ} = \int N_I N_J \quad F_J = \int f N_J$$

$$\Rightarrow \underbrace{M}_{\text{'mass matrix'}} \underbrace{\nu}_{\text{'force vector'}} = \underbrace{F}_{\text{'force vector'}}$$

This can also be obtained from

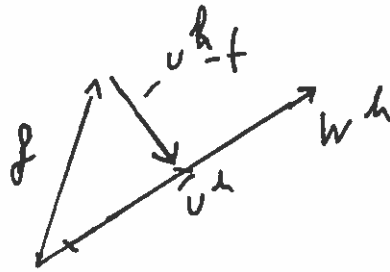
$$\pi = \frac{1}{2} \int \left(\sum_I N_I \nu_I - f \right)^2 \quad \text{'least squares fit'}$$

minimize π

$$\frac{\partial \pi}{\partial \nu_J} = \int \left(\sum_I N_I \nu_I - f \right) N_J = 0$$

$$\int w^h (u^h - f) = 0$$

error orthogonal
to all possible
finite element
test functions



\Rightarrow hence u^h is a projection

c)

$$\int v_{,ii} w_{,i} + \int v c_{,i} w_{,i} + \int (v - f) w = 0$$

$$- \int v_{,ii} w + BT - \int (v c_{,i})_{,i} w + BT \int (v - f) w = 0$$

$$- v_{,ii} - (v c_{,i})_{,i} + v - f = 0$$

1. (a) (i) $-d/dx(E dx/dx) = f$

Multiply by test function v & integrate.

$$-\int_0^L v \frac{d}{dx} \left(E \frac{dx}{dx} \right) dx = \int_0^L v f dx$$

Integrate by parts:

$$\int_0^L \frac{dv}{dx} E \frac{dx}{dx} dx - v \frac{d}{dx} \left(E \frac{dx}{dx} \right) \Big|_0^L = \int_0^L v f dx$$

- Dirichlet b.c. @ $x=0$ $\therefore v(0) = 0$

- Inset Neuman b.c. @ $x=L$

$$\Rightarrow \int_0^L \frac{dv}{dx} E \frac{dx}{dx} dx = \int_0^L v f dx + v h \Big|_{x=L}$$

for all admissible v

$$(ii) \underline{k}_e = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$\underline{f}_e = f \begin{bmatrix} \int_0^L v dx \\ \int_0^L v dx \end{bmatrix} = \frac{f}{2} \begin{bmatrix} L \\ L \end{bmatrix}$$

$$(iii) \underline{f}_e = \int_0^L v f dx$$

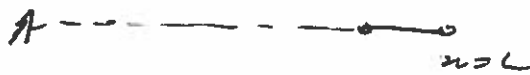
polynomial degree of integral = $n-1$
 \Rightarrow 1 quadrature point for exact integration.

$$(b) (i) \quad + \int_0^L \frac{dW}{dn} E \frac{du}{dn} \quad g - v \frac{E du}{dn} \Big|_{x=L} = \int_0^L v f dx$$

Re-arrang Robin bc:

$$au + E \frac{du}{dn} = g \implies \frac{E du}{dn} = g - au$$

$$\int_0^L \frac{du}{dn} E \frac{du}{dn} + v au \Big|_{x=L} = \int_0^L v f dx + v g \Big|_{x=L}$$



$$(ii) \quad \underline{\underline{K_c}} = \frac{E}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} + a \begin{bmatrix} 0 & 0 \\ 0 & 1 \end{bmatrix}$$

$$\underline{\underline{f_c}} = \frac{fL}{2} \begin{bmatrix} 1 \\ 1 \end{bmatrix} + \begin{bmatrix} 0 \\ g \end{bmatrix}$$

$$(iii) \quad \text{force/stress} = g - au \\ = a \left(\frac{g}{a} - u \right)$$

$\therefore a$ acts as effectively a spring stiffness



Q2

$$(10) \quad \frac{y_{n+1} - y_n}{\Delta t} = -\lambda((1-\theta)y_n + \theta y_{n+1})$$

Rearranging into $y_{n+1} = Ay_n$, where A is the amplification factor:

$$(1 + \lambda \Delta t \theta) y_{n+1} = (1 - \lambda \Delta t (1-\theta)) y_n$$

$$y_{n+1} = \frac{(1 + \lambda \Delta t \theta - \lambda \Delta t)}{(1 + \lambda \Delta t \theta)} y_n$$

A

For stability, $|A| \leq 1$

$$A = \frac{1 + \lambda \Delta t \theta}{1 + \lambda \Delta t \theta} - \frac{\lambda \Delta t}{1 + \lambda \Delta t \theta}$$

= 1

must be < 2 for all Δt
for $|A| \leq 1$

$$\Rightarrow \theta \geq \frac{1}{2} \Rightarrow < 2 \text{ for all } \Delta t$$

$$(b)(i) \quad \underline{M} \underline{\dot{u}} + \underline{K} \underline{u} = \underline{f}$$

$$\underline{M} \left(\frac{\underline{u}_{n+1} - \underline{u}_n}{\Delta t} \right) + \underline{K} (\theta(1-\theta)\underline{u}_n + \theta \underline{u}_{n+1}) = \underline{f}_{n+\theta}$$

$$\underline{M}(\underline{u}_{n+1}) - \underline{M}\underline{u}_n + \Delta t \underline{K} \underline{u}_n + \Delta t \theta \underline{K} \underline{u}_{n+1} = \Delta t \underline{f}_{n+\theta}$$

$$(\underline{M} + \theta \Delta t \underline{K}) \underline{u}_{n+1} = \Delta t \underline{f}_{n+\theta} + \underline{M} \underline{u}_n - \Delta t (1-\theta) \underline{K} \underline{u}_n$$

$$\hat{\underline{M}} \underline{u}_{n+1} = \hat{\underline{F}}$$

(ii) Modal analysis reduces two dot problem to two eqns with same form as modal problem, where λ is an eigenvalue of:

$$\underline{K} \underline{x} = \lambda \underline{M} \underline{x}$$

To find λ :

$$(\underline{K} - \lambda \underline{M}) \underline{x} = \underline{0}$$

$$\Rightarrow \det(\underline{K} - \lambda \underline{M}) = 0$$

$$|\underline{K}'| = \begin{vmatrix} 2 - 6\lambda & -1 - \lambda \\ -1 - \lambda & 1 - 5\lambda \end{vmatrix}$$

$$= (2 - 6\lambda)(1 - 5\lambda) - (-1 - \lambda)^2$$

$$= 2 - 16\lambda + 30\lambda^2 - (1 + 2\lambda + \lambda^2)$$

$$= 1 - 18\lambda + 29\lambda^2$$

Find roots:

$$\frac{18 \pm \sqrt{18^2 - 4(29)}}{2(29)} = \frac{18 \pm \sqrt{208}}{58}$$

$$\Delta t_{\text{crit}} = \frac{2}{\lambda} \quad \therefore \lambda_{\text{max}} \text{ determ. step}$$

$$\Delta t_{\text{crit}} = 2/\lambda_{\text{max}} \approx 2(58) / (18 + \sqrt{208})$$

$$\approx \underline{3.58}$$

(iii) When $\alpha < 1/2$, critid time step is proportional to $1/\lambda_{\text{max}}$. $\lambda_{\text{max}} \propto 1/h^2$ (cell size)

$$\therefore \Delta t_{\text{crit}} \propto h^2$$

This is often too restrictive for practical use, therefore an implicit scheme is often preferred.