

ENGINEERING TRIPOS PART IB

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Monday 4 June 2007 2 to 4

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Paper 2

STRUCTURES

*Answer not more than **four** questions, which may be taken from either section.*

*All questions carry the same number of marks.*

*The **approximate** number of marks allocated to each part of a question is indicated in the right margin.*

*Answers to questions in each section should be tied together and handed in separately.*

*There are no attachments.*

STATIONERY REQUIREMENTS  
Single-sided script paper

SPECIAL REQUIREMENTS  
Engineering Data Book  
CUED approved calculator allowed

<p><b>You may not start to read the questions printed on the subsequent pages of this question paper until instructed that you may do so by the Invigilator.</b></p>
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## SECTION A

1 The pin-jointed structure in Fig. 1 has linear elastic bars of equal cross-sectional area  $A$ , made of the same material of Young's modulus  $E$ . There are no initial stresses and self-weight is to be ignored.

(a) Determine the number of redundancies. [2]

(b) Using the Force Method, determine a set of bar forces in equilibrium with the applied forces,  $H$  and  $V$ , as shown. Compute the actual bar forces in the structure. [14]

(c) Find the components of displacement of the pin-joint at which the loads are applied. [4]

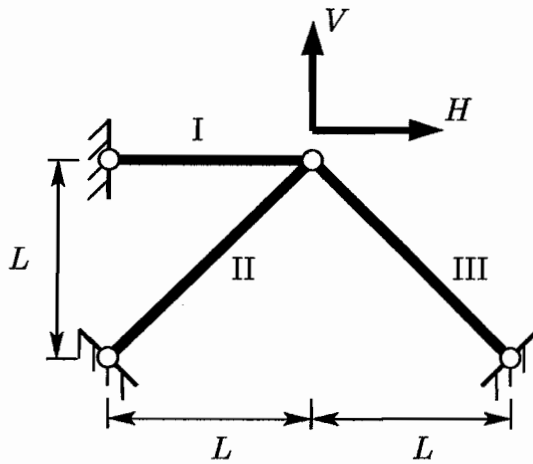


Fig. 1

2 (a) Figure 2 shows two different thin-walled cross-sections of the dimensions indicated. They are made of the same plate material, of thickness  $t$ . With respect to the square cross-section, show that the ratio of torsional stiffness of the square cross-section to the H-cross-section is  $1:(49/108)$ , and that the ratio of bending stiffness about a horizontal axis is  $1:(55/54)$ . [7]

(b) A thin-walled box section has a square cross-section of side-length  $b$  and wall-thickness  $t$ , and carries a bending moment about a horizontal axis  $M$  and a torque  $T$ . For this loading, draw the Mohr's circles of stress for an element of material in the top flange, assuming that  $M$  results in a positive moment. Determine the relationships between  $M$  and  $T$  at first yield according to (i) Tresca's criterion and (ii) von-Mises' criterion. [13]

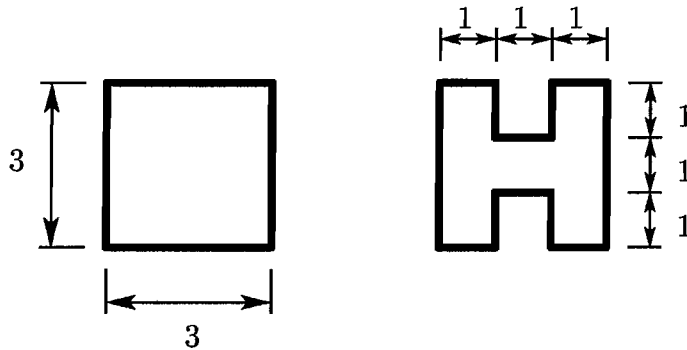


Fig. 2

(TURN OVER)

3 (a) The planar quadrant rod AB shown in Fig. 3(a) undergoes a uniform rise in temperature  $T$  above ambient conditions. The rod has a uniform cross-section and is initially unstressed. The linear elastic bending stiffness is  $EI$  and the linear coefficient of thermal expansion is  $\alpha$ .

(i) If the rod end B is initially unconstrained, show that it undergoes a vertical displacement of  $\alpha RT$ . [4]

(ii) The rod end B is now constrained vertically before heating, as shown in Fig. 3(a). Determine the reaction at the constraint after heating. [10]

(b) An isolated ring, shown in Fig. 3(b), experiences a *variable* rise in temperature  $T = T_0 \sin \theta$  where  $\theta$  is the angle subtended around the ring. Note that  $T$  is uniform through the thickness of rod. Find the maximum bending moment. [6]

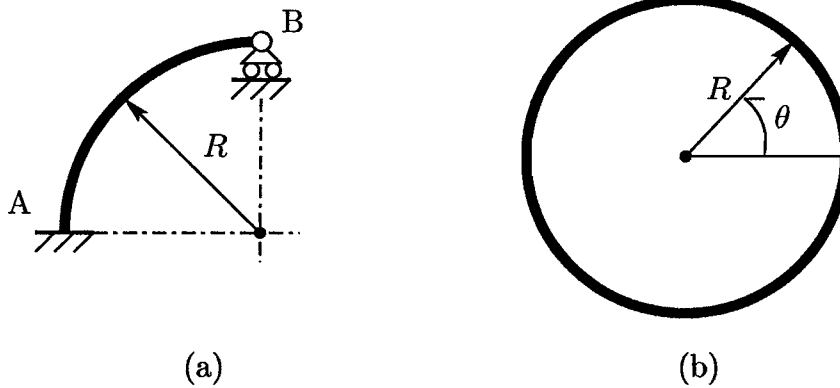


Fig. 3

## SECTION B

4 The structures shown in Figs. 4(a) - (c) have a uniform fully-plastic moment  $M_p$  and are unstressed when unloaded. All forces are vertical and are applied half-way between the supports, as indicated.

(a) Perform a lower bound analysis to determine the maximum, safe value of  $F$  for:

(i) the propped cantilever in Fig. 4(a); [3]

(ii) the cantilever in Fig. 4(b), whose tip can deflect but not rotate. [3]

(b) The four-span continuous beam in Fig. 4(c) is interrupted by a pin over the middle support. Perform a lower bound analysis to find the maximum safe load when:

(i)  $F_1 = F_2 = F$ ; [4]

(ii)  $F_1 = F$ ,  $F_2 = -F$ . [4]

(c) When  $F_1 = F_2 = F$ , and the pin is replaced by a continuous connection, find the maximum safe value of  $F$ . [6]

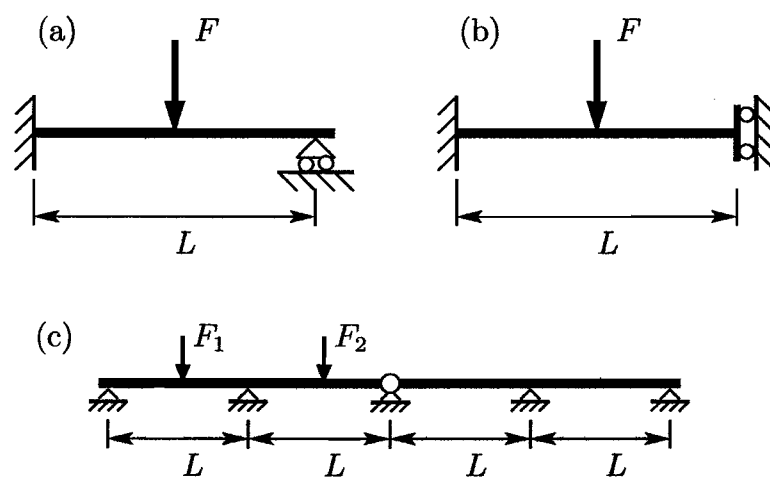


Fig. 4

(TURN OVER)

5 A stepped frame ABCD carries two loads  $H$  and  $V$  as shown in Fig. 5. The fully plastic moment is  $M_p$  in the longer beams BC and CD, and  $\lambda M_p$  in the shorter beam AB where  $0 < \lambda < 1$ . The frame is initially unstressed and deforms within its own plane.

(a) Perform an upper bound analysis to determine the collapse conditions for:

- (i) a beam mechanism located on AB; [3]
- (ii) a beam mechanism located on BC; [3]
- (iii) one sway mechanism; [3]
- (iv) one combined mechanism with four plastic hinges. [3]

For  $\lambda = 0.5$ , combine all results into a single interaction diagram. [2]

(b) By considering a combined mechanism with *three* plastic hinges, determine the corresponding collapse conditions for all values of  $H$  and  $V$ . [6]

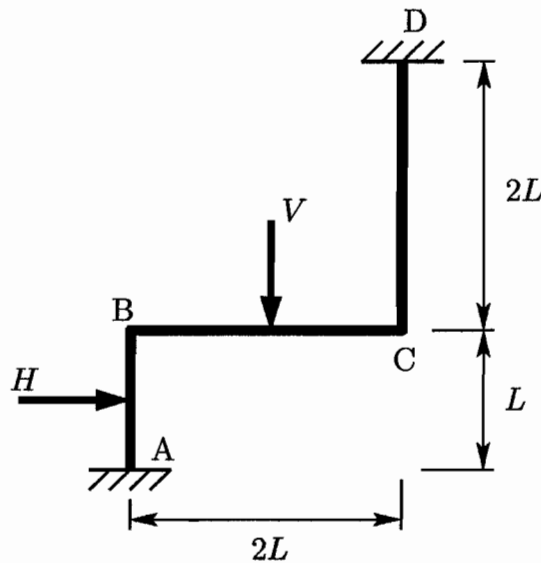


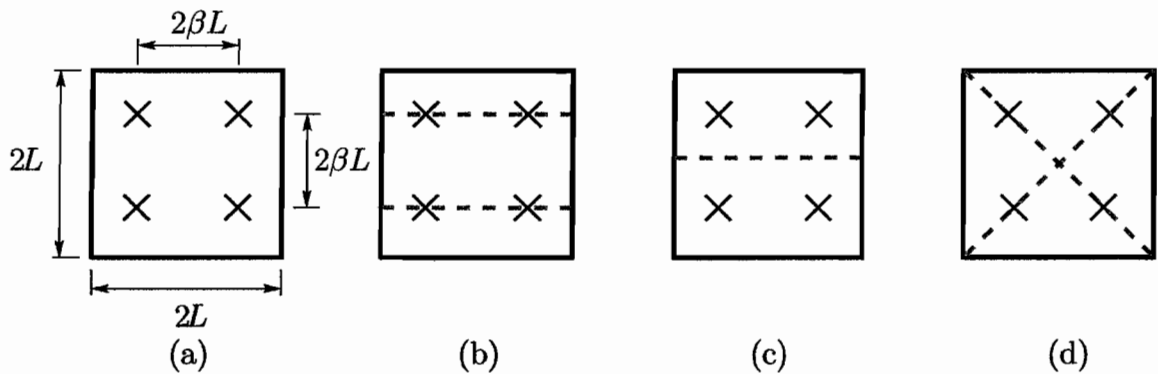
Fig. 5

6 The square slab in Fig. 6(a) has side-length  $2L$  and is shown in plan view. It carries a uniformly distributed pressure loading  $p$  and is supported on four *internal* columns, arranged in a square layout of side-length  $2\beta L$  of the same centre as the slab. Each column performs as a point support. The slab is initially unstressed and has uniform moment of resistance per length  $m$  both for hogging and sagging.

(a) By considering the collapse mechanism of two yield lines shown in Fig. 6(b), find the critical value of  $p$ . [6]

(b) By considering the collapse mechanism of a single yield line shown in Fig. 6(c), find the critical value of  $p$  and find the value of  $\beta$  which results in a smaller collapse load compared to the result found in (a). [6]

(c) By considering the collapse mechanism shown in Fig. 6(d), show that the critical value of  $p$  cannot be improved compared to the results found in (a) and (b). [8]



Note: cracks are shown as dashed lines

Fig. 6

**END OF PAPER**