

ENGINEERING TRIPOS

PART IIB

Friday 28 April 2006

9.00 to 10.30

Module 4C1

DESIGN AGAINST FAILURE

*Answer not more than **three** questions.*

All questions carry the same number of marks.

*The **approximate** percentage of marks allocated to each part of a question is indicated in the right margin.*

Attachments:

Elasticity and Plasticity formulae (2 pages)

Fracture Mechanics Datasheet (8 pages)

STATIONERY REQUIREMENTS

Single-sided script paper

SPECIAL REQUIREMENTS

Engineering Data Book

CUED approved calculator allowed

**You may not start to read the questions
printed on the subsequent pages of this
question paper until instructed that you may
do so by the Invigilator**

- 1 (a) Define line tension T along a dislocation line and relate it to the dislocation line energy. Comment on why a dislocation line tends to be straight in the absence of stresses. [15%]
- (b) Explain how to determine the magnitude of a typical Burger's vector. [10%]
- (c) Define the force acting on a dislocation. [10%]
- (d) Derive an expression for the ideal shear strength, and hence explain why metals do not typically achieve this ideal strength. [25%]
- (e) Explain the operation of a Frank-Read dislocation source, and hence derive an expression for the yield stress in shear. Using estimates for the relevant quantities, calculate the value of the shear yield strength of a typical metal. Comment on how this compares with the ideal shear strength. [25%]
- (f) Assuming that the pinning points in the Frank-Read model are dislocation jogs, comment on how the shear yield strength derived with this model compares with the ideal shear strength. [15%]

2 (a) Sketch the uniaxial tensile stress versus strain curve of an amorphous polymer tested *well below* $0.8T_g$ where T_g is the glass transition temperature, and describe the underlying failure mechanisms. [15%]

(b) Sketch the uniaxial tensile stress versus strain curve of an amorphous polymer tested *at* $0.8T_g$, and describe the underlying failure mechanisms. [15%]

(c) Construct a failure mechanism map for an amorphous polymer. [20%]

(d) Describe briefly what is meant by Coble creep and Nabarro-Herring creep. [20%]

(e) Figure 1 shows a cylindrical volume element subjected to axisymmetric loading, with axial stress σ_1 and radial stress σ_2 . For each of the following room temperature deformation and/or failure processes, sketch the failure surfaces in axisymmetric stress space.

(i) Plastic yielding; [15%]

(ii) Cleavage failure (propagation controlled). [15%]

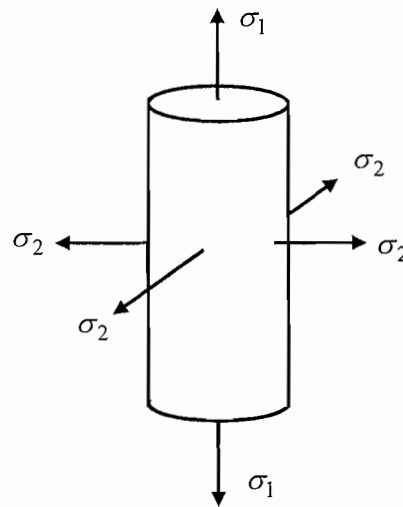


Fig. 1

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3 A semi-infinite solid contains a thin brittle interface layer parallel to the surface at a depth a as sketched in Fig. 2. The bulk properties of the solid are identical on either side of the layer. A surface crack extends normal to the surface, with its tip at the interface as shown in Fig. 2. Remote loading may be applied to generate either a mode I stress intensity K_I or a mode II stress intensity K_{II} . The stress components $\sigma_{\theta\theta}$ and $\sigma_{r\theta}$ are

$$\sigma_{\theta\theta} = \frac{1}{\sqrt{2\pi r}} \cos \frac{\theta}{2} \left(K_I \cos^2 \frac{\theta}{2} - \frac{3}{2} K_{II} \sin \theta \right)$$

$$\sigma_{r\theta} = \frac{1}{2\sqrt{2\pi r}} \cos \frac{\theta}{2} \left(K_I \cos^2 \frac{\theta}{2} + K_{II} (3 \cos \theta - 1) \right)$$

where the polar coordinates (r, θ) are defined in Fig. 2. It may be assumed that the crack in Fig. 2 propagates in the bulk when the maximum value of $\sigma_{\theta\theta}$ at a distance $r = 0.01a$ reaches a critical value ρ_A . Failure of the interface may be assumed to occur when the stress normal to the interface at a distance $r = 0.01a$ reaches a critical value ρ_B .

(a) For the case of remote mode I loading, derive a criterion to determine whether cracking occurs along the interface or in the bulk, in terms of the ratio ρ_A / ρ_B . [40%]

(b) The crack is now loaded in pure mode II.

(i) Find the direction in which the crack would propagate in the bulk. [20%]

(ii) Derive the criterion to determine whether the crack branches into the interface or into the bulk, in terms of ρ_A / ρ_B . [40%]

(contd.)

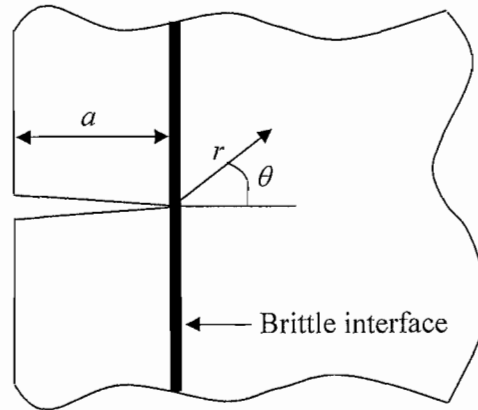


Fig. 2

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4 (a) Explain why the critical energy release rate for fracture, G_{IC} , is much greater than twice the surface energy for engineering alloys. [15%]

(b) A flat circular indenter of radius a is pressed against the surface of a brittle semi-infinite solid with a load P , as shown in the cross-section in Fig. 3. A cylindrical crack of length l forms in the solid, initiating at the edge of the indenter.

(i) Show that the compliance C is approximately given by

$$C = \frac{l}{E\pi a^2}$$

where E is the Young's modulus for the solid. [15%]

(ii) Find an expression for the strain energy release rate G for the cylindrical crack under the indenter, and comment on the stability of crack advance. [50%]

(iii) Briefly discuss whether you would expect the cylindrical crack to deviate from a constant radius with increasing depth. [20%]

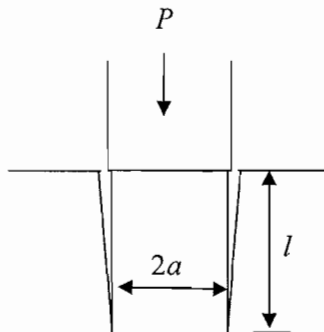


Fig. 3

END OF PAPER

Paper G4: Mechanics of Solids
ELASTICITY and PLASTICITY FORMULAE

1. Axi-symmetric deformation : discs, tubes and spheres

	<u>Discs and tubes</u>	<u>Spheres</u>
Equilibrium	$\sigma_{\theta\theta} = \frac{d(r\sigma_r)}{dr} + \rho\omega^2 r^2$	$\sigma_{\theta\theta} = \frac{1}{2r} \frac{d(r^2\sigma_r)}{dr}$
Lamé's equations (in elasticity)	$\sigma_r = A - \frac{B}{r^2} - \frac{3+\nu}{8} \rho\omega^2 r^2 - \frac{E\alpha}{r^2} \int_r^c rTdr$	$\sigma_r = A - \frac{B}{r^3}$
	$\sigma_{\theta\theta} = A + \frac{B}{r^2} - \frac{1+3\nu}{8} \rho\omega^2 r^2 + \frac{E\alpha}{r^2} \int_r^c rTdr - E\alpha T$	$\sigma_{\theta\theta} = A + \frac{B}{2r^3}$

2. Plane stress and plane strain

	<u>Cartesian coordinates</u>	<u>Polar coordinates</u>
Strains	$\epsilon_{xx} = \frac{\partial u}{\partial x}$ $\epsilon_{yy} = \frac{\partial v}{\partial y}$ $\gamma_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}$	$\epsilon_r = \frac{\partial u}{\partial r}$ $\epsilon_{\theta\theta} = \frac{u}{r} + \frac{1}{r} \frac{\partial v}{\partial \theta}$ $\gamma_{r\theta} = \frac{\partial v}{\partial r} + \frac{1}{r} \frac{\partial u}{\partial \theta} - \frac{v}{r}$
Compatibility	$\frac{\partial^2 \gamma_{xy}}{\partial x \partial y} = \frac{\partial^2 \epsilon_{xx}}{\partial y^2} + \frac{\partial^2 \epsilon_{yy}}{\partial x^2}$	$\frac{\partial}{\partial r} \left\{ r \frac{\partial \gamma_{r\theta}}{\partial \theta} \right\} = \frac{\partial}{\partial r} \left\{ r^2 \frac{\partial \epsilon_{\theta\theta}}{\partial r} \right\} - r \frac{\partial \epsilon_r}{\partial r} + \frac{\partial^2 \epsilon_r}{\partial \theta^2}$
or (in elasticity)	$\left\{ \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2} \right\} (\sigma_{xx} + \sigma_{yy}) = 0$	$\left\{ \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{1}{r^2} \frac{\partial^2}{\partial \theta^2} \right\} (\sigma_r + \sigma_{\theta\theta}) = 0$
Equilibrium	$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} = 0$ $\frac{\partial \sigma_{yy}}{\partial y} + \frac{\partial \sigma_{xy}}{\partial x} = 0$	$\frac{\partial}{\partial r} (r\sigma_r) + \frac{\partial \sigma_{r\theta}}{\partial \theta} - \sigma_{\theta\theta} = 0$ $\frac{\partial \sigma_{\theta\theta}}{\partial \theta} + \frac{\partial}{\partial r} (r\sigma_{r\theta}) + \sigma_{r\theta} = 0$
$\nabla^4 \phi = 0$ (in elasticity)	$\left\{ \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2} \right\} \left\{ \frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} \right\} = 0$	$\left\{ \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{1}{r^2} \frac{\partial^2}{\partial \theta^2} \right\}$ $\times \left\{ \frac{\partial^2 \phi}{\partial r^2} + \frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} \right\} = 0$
Airy Stress Function	$\sigma_{xx} = \frac{\partial^2 \phi}{\partial y^2}$ $\sigma_{yy} = \frac{\partial^2 \phi}{\partial x^2}$ $\sigma_{xy} = -\frac{\partial^2 \phi}{\partial x \partial y}$	$\sigma_r = \frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2}$ $\sigma_{\theta\theta} = \frac{\partial^2 \phi}{\partial r^2}$ $\sigma_{r\theta} = -\frac{\partial}{\partial r} \left[\frac{1}{r} \frac{\partial \phi}{\partial \theta} \right]$

3. Torsion of prismatic bars

Prandtl stress function: $\sigma_{zx} (= \tau_x) = \frac{dF}{dy}$, $\sigma_{zy} (= \tau_y) = -\frac{dF}{dx}$

Equilibrium: $T = 2 \int_A F dA$

Governing equation for elastic torsion: $\nabla^2 F = -2G\beta$ where β is the angle of twist per unit length.

4. Total potential energy of a body

$$\Pi = U - W$$

where $U = \frac{1}{2} \int_V \underline{\underline{\varepsilon}}^T [D] \underline{\underline{\varepsilon}} dV$, $W = \underline{\underline{p}}^T \underline{\underline{u}}$ and $[D]$ is the elastic stiffness matrix.

5. Principal stresses and stress invariants

Values of the principal stresses, σ_p , can be obtained from the equation

$$\begin{vmatrix} \sigma_{xx} - \sigma_p & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} - \sigma_p & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} - \sigma_p \end{vmatrix} = 0$$

This is equivalent to a cubic equation whose roots are the values of the 3 principal stresses, i.e. the possible values of σ_p .

Expanding: $\sigma_p^3 - I_1 \sigma_p^2 + I_2 \sigma_p - I_3 = 0$ where $I_1 = \sigma_{xx} + \sigma_{yy} + \sigma_{zz}$,

$$I_2 = \begin{vmatrix} \sigma_{yy} & \sigma_{yz} \\ \sigma_{yz} & \sigma_{zz} \end{vmatrix} + \begin{vmatrix} \sigma_{xx} & \sigma_{xz} \\ \sigma_{xz} & \sigma_{zz} \end{vmatrix} + \begin{vmatrix} \sigma_{xx} & \sigma_{xy} \\ \sigma_{xy} & \sigma_{yy} \end{vmatrix} \quad \text{and} \quad I_3 = \begin{vmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{xy} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} \end{vmatrix}.$$

6. Equivalent stress and strain

Equivalent stress $\bar{\sigma} = \sqrt{\frac{1}{2} \{ (\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2 \}}^{1/2}$

Equivalent strain increment $d\bar{\varepsilon} = \sqrt{\frac{2}{3} \{ d\varepsilon_1^2 + d\varepsilon_2^2 + d\varepsilon_3^2 \}}^{1/2}$

7. Yield criteria and flow rules

Tresca

Material yields when maximum value of $|\sigma_1 - \sigma_2|$, $|\sigma_2 - \sigma_3|$ or $|\sigma_3 - \sigma_1| = Y = 2k$, and then,

if σ_3 is the intermediate stress, $d\varepsilon_1 : d\varepsilon_2 : d\varepsilon_3 = \lambda(1 : -1 : 0)$ where $\lambda \neq 0$.

von Mises

Material yields when, $(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2 = 2Y^2 = 6k^2$, and then

$$\frac{d\varepsilon_1}{\sigma_1} = \frac{d\varepsilon_2}{\sigma_2} = \frac{d\varepsilon_3}{\sigma_3} = \frac{d\varepsilon_1 - d\varepsilon_2}{\sigma_1 - \sigma_2} = \frac{d\varepsilon_2 - d\varepsilon_3}{\sigma_2 - \sigma_3} = \frac{d\varepsilon_3 - d\varepsilon_1}{\sigma_3 - \sigma_1} = \lambda = \frac{3}{2} \frac{d\bar{\varepsilon}}{\bar{\sigma}}.$$

B1 DEFORMATION AND FRACTURE

FRACTURE MECHANICS DATASHEET

Crack tip plastic zone sizes

$$\text{diameter, } d_p = \begin{cases} \frac{1}{\pi} \left(\frac{K_I}{\sigma_y} \right)^2 & \text{Plane stress} \\ \frac{1}{3\pi} \left(\frac{K_I}{\sigma_y} \right)^2 & \text{Plane strain} \end{cases}$$

Crack opening displacement

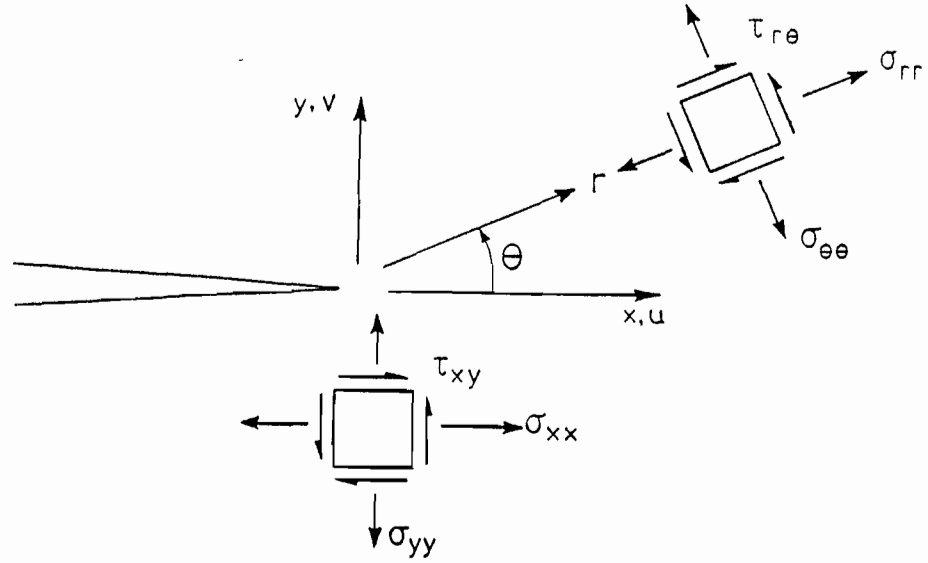
$$\delta = \begin{cases} \frac{K_I^2}{\sigma_y E} & \text{Plane stress} \\ \frac{1}{2} \frac{K_I^2}{\sigma_y E} & \text{Plane strain} \end{cases}$$

Energy release rate

$$G = \begin{cases} \frac{1}{E} K_I^2 & \text{Plane stress} \\ \frac{1-\nu^2}{E} K_I^2 & \text{Plane strain} \end{cases}$$

Related to compliance C : $G = \frac{1}{2} \frac{P^2}{B} \frac{dC}{da}$

Asymptotic crack tip fields in a linear elastic solid



Mode I

$$\sigma_{yy} = \frac{K_I}{\sqrt{2\pi r}} \cos \frac{\theta}{2} \left(1 + \sin \frac{\theta}{2} \sin \frac{3\theta}{2} \right)$$

$$\sigma_{xx} = \frac{K_I}{\sqrt{2\pi r}} \cos \frac{\theta}{2} \left(1 - \sin \frac{\theta}{2} \sin \frac{3\theta}{2} \right)$$

$$\tau_{xy} = \frac{K_I}{\sqrt{2\pi r}} \cos \frac{\theta}{2} \sin \frac{\theta}{2} \cos \frac{3\theta}{2}$$

$$\sigma_{rr} = \frac{K_I}{\sqrt{2\pi r}} \left(\frac{5}{4} \cos \frac{\theta}{2} - \frac{1}{4} \cos \frac{3\theta}{2} \right)$$

$$\sigma_{\theta\theta} = \frac{K_I}{\sqrt{2\pi r}} \left(\frac{3}{4} \cos \frac{\theta}{2} + \frac{1}{4} \cos \frac{3\theta}{2} \right)$$

$$\tau_{r\theta} = \frac{K_I}{\sqrt{2\pi r}} \left(\frac{1}{4} \sin \frac{\theta}{2} + \frac{1}{4} \sin \frac{3\theta}{2} \right)$$

$$u = \begin{cases} \frac{K_I}{G} \sqrt{\frac{r}{2\pi}} \left(\frac{1-\nu}{1+\nu} + \sin^2 \frac{\theta}{2} \right) \cos \frac{\theta}{2} & \text{Plane stress} \\ \frac{K_I}{G} \sqrt{\frac{r}{2\pi}} \left(1 - 2\nu + \sin^2 \frac{\theta}{2} \right) \cos \frac{\theta}{2} & \text{Plane strain} \end{cases}$$

$$v = \begin{cases} \frac{K_I}{G} \sqrt{\frac{r}{2\pi}} \left(\frac{2}{1+\nu} - \cos^2 \frac{\theta}{2} \right) \sin \frac{\theta}{2} & \text{Plane stress} \\ \frac{K_I}{G} \sqrt{\frac{r}{2\pi}} \left(2 - 2\nu - \cos^2 \frac{\theta}{2} \right) \sin \frac{\theta}{2} & \text{Plane strain} \end{cases}$$

$$w = 0$$

Crack tip stress fields (cont'd)

Mode II

$$\sigma_{yy} = \frac{K_{II}}{\sqrt{2\pi r}} \cos \frac{\theta}{2} \sin \frac{\theta}{2} \cos \frac{3\theta}{2}$$

$$\sigma_{xx} = -\frac{K_{II}}{\sqrt{2\pi r}} \sin \frac{\theta}{2} \left(2 + \cos \frac{\theta}{2} \cos \frac{3\theta}{2} \right)$$

$$\tau_{xy} = \frac{K_{II}}{\sqrt{2\pi r}} \cos \frac{\theta}{2} \left(1 - \sin \frac{\theta}{2} \sin \frac{3\theta}{2} \right)$$

$$\sigma_{rr} = \frac{K_{II}}{\sqrt{2\pi r}} \left(-\frac{5}{4} \sin \frac{\theta}{2} + \frac{3}{4} \sin \frac{3\theta}{2} \right)$$

$$\sigma_{\theta\theta} = -\frac{K_{II}}{\sqrt{2\pi r}} \left(\frac{3}{4} \sin \frac{\theta}{2} + \frac{3}{4} \sin \frac{3\theta}{2} \right)$$

$$\tau_{r\theta} = \frac{K_{II}}{\sqrt{2\pi r}} \left(\frac{1}{4} \cos \frac{\theta}{2} + \frac{3}{4} \cos \frac{3\theta}{2} \right)$$

$$u = \begin{cases} \frac{K_{II}}{G} \sqrt{\frac{r}{2\pi}} \left(\frac{2}{1+\nu} + \cos^2 \frac{\theta}{2} \right) \sin \frac{\theta}{2} & \text{Plane stress} \\ \frac{K_{II}}{G} \sqrt{\frac{r}{2\pi}} \left(2 - 2\nu + \cos^2 \frac{\theta}{2} \right) \sin \frac{\theta}{2} & \text{Plane strain} \end{cases}$$

$$v = \begin{cases} \frac{K_{II}}{G} \sqrt{\frac{r}{2\pi}} \left(\frac{\nu-1}{1+\nu} + \sin^2 \frac{\theta}{2} \right) \cos \frac{\theta}{2} & \text{Plane stress} \\ \frac{K_{II}}{G} \sqrt{\frac{r}{2\pi}} \left(-1 + 2\nu + \sin^2 \frac{\theta}{2} \right) \cos \frac{\theta}{2} & \text{Plane strain} \end{cases}$$

$$w = 0$$

Mode III

$$\tau_{zx} = -\frac{K_{III}}{\sqrt{2\pi r}} \sin \frac{\theta}{2}$$

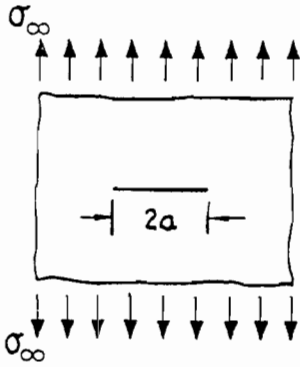
$$\tau_{yz} = \frac{K_{III}}{\sqrt{2\pi r}} \cos \frac{\theta}{2}$$

$$\sigma_{xx} = \sigma_{yy} = \sigma_{zz} = \tau_{xy} = 0$$

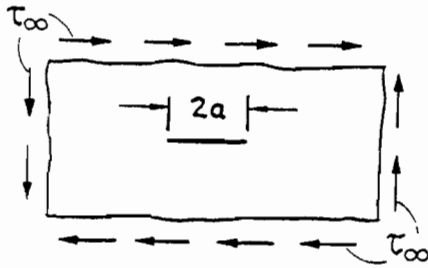
$$w = \frac{K_{III}}{G} \sqrt{\frac{2r}{\pi}} \sin \frac{\theta}{2}$$

$$u = v = 0$$

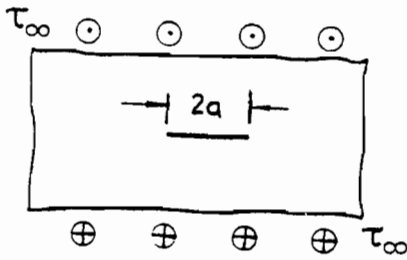
Tables of stress intensity factors



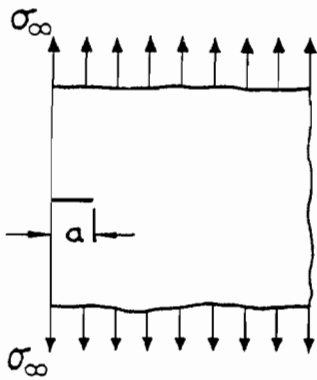
$$K_I = \sigma_\infty \sqrt{\pi a}$$



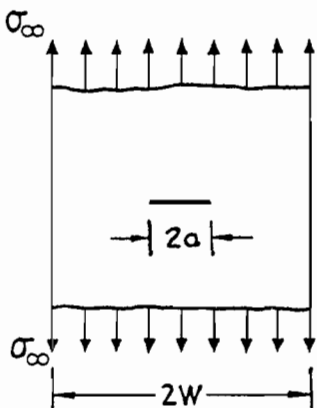
$$K_{II} = \tau_\infty \sqrt{\pi a}$$



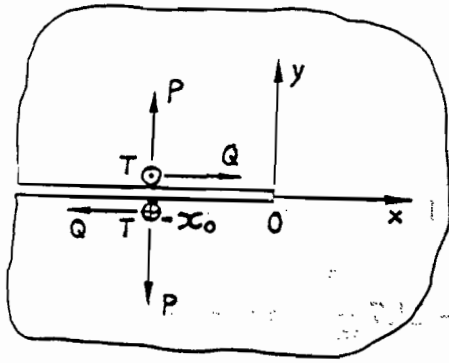
$$K_{III} = \tau_\infty \sqrt{\pi a}$$



$$K_I = 1.12 \sigma_\infty \sqrt{\pi a}$$



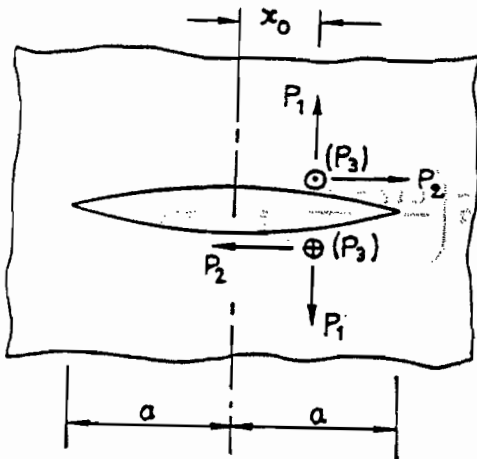
$$K_I = \sigma_\infty \sqrt{\pi a} \left(\frac{1 - a/2W + 0.326a^2/W^2}{\sqrt{1 - a/W}} \right)$$



$$K_I = \frac{2P}{\sqrt{2\pi x_0}}$$

$$K_{II} = \frac{2Q}{\sqrt{2\pi x_0}}$$

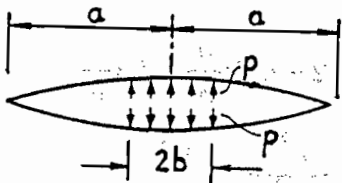
$$K_{III} = \frac{2T}{\sqrt{2\pi x_0}}$$



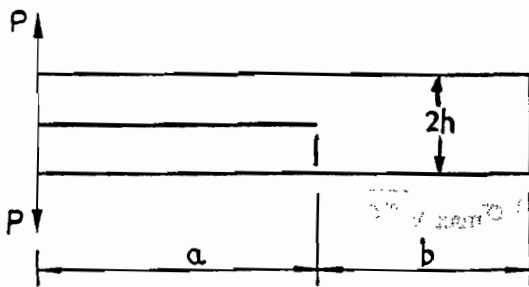
$$K_I = \frac{P_1}{\sqrt{\pi a}} \sqrt{\frac{a+x_0}{a-x_0}}$$

$$K_{II} = \frac{P_2}{\sqrt{\pi a}} \sqrt{\frac{a+x_0}{a-x_0}}$$

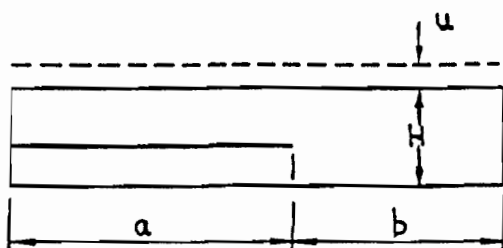
$$K_{III} = \frac{P_3}{\sqrt{\pi a}} \sqrt{\frac{a+x_0}{a-x_0}}$$



$$K_I = \frac{2pb}{\sqrt{\pi a}} \frac{a}{b} \arcsin \frac{b}{a}$$

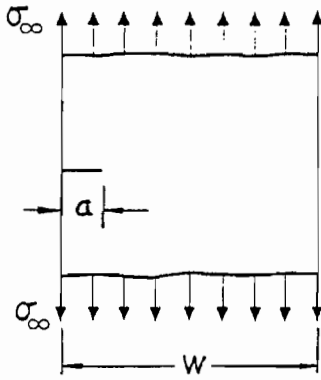


$$K_I = \frac{2\sqrt{3}}{h\sqrt{h}} \frac{Pa}{B} \quad h \ll a \text{ and } h \ll b$$



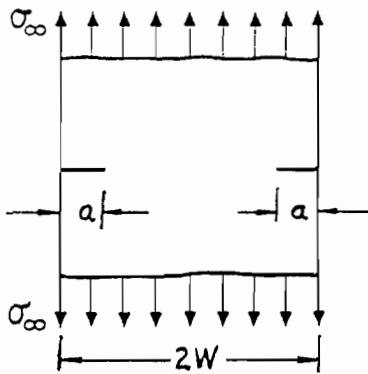
$$K_I = \sqrt{\frac{1}{2\alpha H}} Eu \quad H \ll a \text{ and } H \ll b$$

$$\alpha = \begin{cases} 1 - \nu^2 & \text{Plane stress} \\ 1 - 3\nu^2 - 2\nu^3 & \text{Plane strain} \end{cases}$$

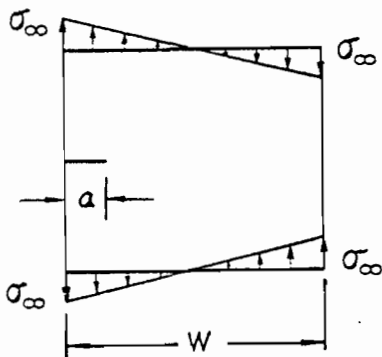


$$a/W < 0.7$$

$$K_I = \sigma_{\infty} \sqrt{\pi a} \left(1.12 - 0.23 \frac{a}{W} + 10.6 \frac{a^2}{W^2} - 21.7 \frac{a^3}{W^3} + 30.4 \frac{a^4}{W^4} \right)$$

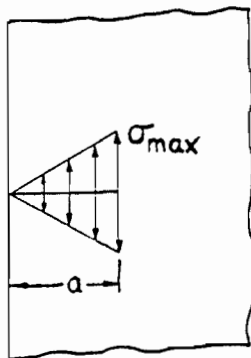


$$K_I = \sigma_{\infty} \sqrt{\pi a} \left(\frac{1.12 - 0.61a/W + 0.13a^3/W^3}{\sqrt{1 - a/W}} \right)$$

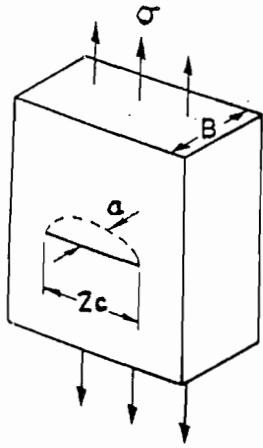


$$a/W < 0.7$$

$$K_I = \sigma_{\infty} \sqrt{\pi a} \left(1.12 - 1.39 \frac{a}{W} + 7.3 \frac{a^2}{W^2} - 13 \frac{a^3}{W^3} + 14 \frac{a^4}{W^4} \right)$$

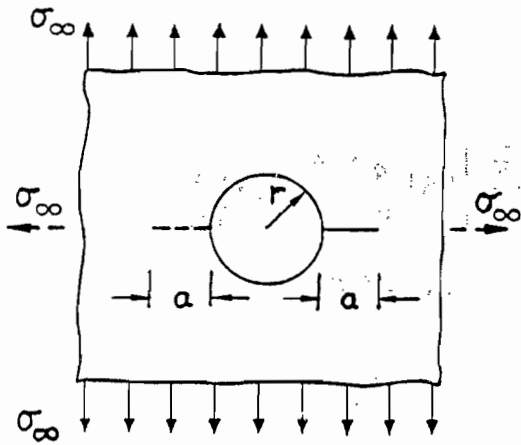
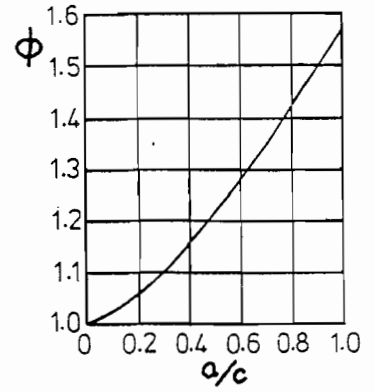


$$K_I = 0.683 \sigma_{\max} \sqrt{\pi a}$$



$$K_I = \frac{1.12}{\Phi} \sigma \sqrt{\pi a}$$

$$\Phi = \int_0^{\pi/2} \left(1 - \frac{c^2 - a^2}{c^2} \sin^2 \theta \right)^{\frac{1}{2}} d\theta$$

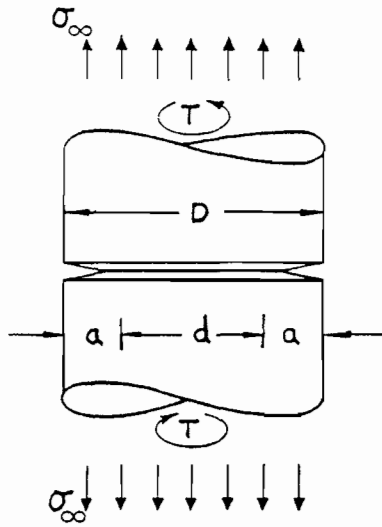


$$K_I = \sigma_{\infty} \sqrt{\pi a} F\left(\frac{a}{r}\right)$$

value of $F(a/r)^\dagger$

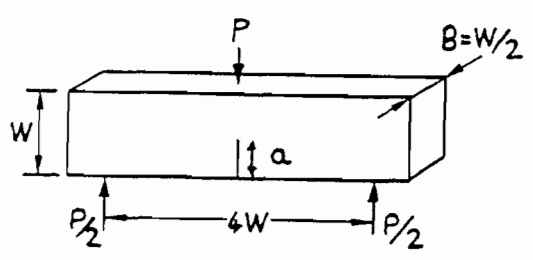
$\frac{a}{r}$	One crack		Two cracks	
	U	B	U	B
0.00	3.36	2.24	3.36	2.24
0.10	2.73	1.98	2.73	1.98
0.20	2.30	1.82	2.41	1.83
0.30	2.04	1.67	2.15	1.70
0.40	1.86	1.58	1.96	1.61
0.50	1.73	1.49	1.83	1.57
0.60	1.64	1.42	1.71	1.52
0.80	1.47	1.32	1.58	1.43
1.0	1.37	1.22	1.45	1.38
1.5	1.18	1.06	1.29	1.26
2.0	1.06	1.01	1.21	1.20
3.0	0.94	0.93	1.14	1.13
5.0	0.81	0.81	1.07	1.06
10.0	0.75	0.75	1.03	1.03
∞	0.707	0.707	1.00	1.00

$^\dagger U = \text{uniaxial } \sigma_{\infty} \quad B = \text{biaxial } \sigma_{\infty}$.

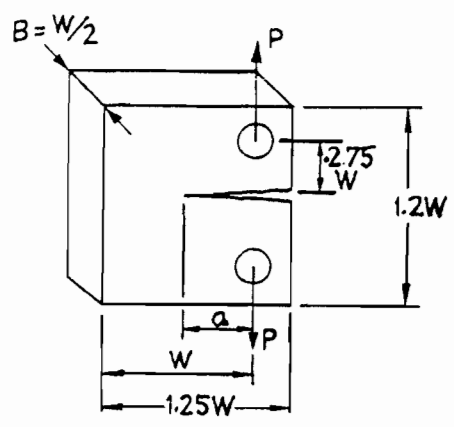


$$K_I = \sigma_{\infty} \sqrt{\pi a} \left(\frac{D}{d} + \frac{1}{2} + \frac{3d}{8D} - 0.36 \frac{d^2}{D^2} + 0.73 \frac{d^3}{D^3} \right) \frac{1}{2} \sqrt{\frac{D}{d}}$$

$$K_{III} = \frac{16T}{\pi D^3} \sqrt{\pi a} \left(\frac{D^2}{d^2} + \frac{1}{2} \frac{D}{d} + \frac{3}{8} + \frac{5}{16} \frac{d}{D} + \frac{35}{128} \frac{d^2}{D^2} + 0.21 \frac{d^3}{D^3} \right) \frac{3}{8} \sqrt{\frac{D}{d}}$$



$$K_I = \frac{4P}{B} \sqrt{\frac{\pi}{W}} \left\{ 1.6 \left(\frac{a}{W} \right)^{1/2} - 2.6 \left(\frac{a}{W} \right)^{3/2} + 12.3 \left(\frac{a}{W} \right)^{5/2} - 21.2 \left(\frac{a}{W} \right)^{7/2} + 21.8 \left(\frac{a}{W} \right)^{9/2} \right\}$$



$$K_I = \frac{P}{B} \sqrt{\frac{\pi}{W}} \left\{ 16.7 \left(\frac{a}{W} \right)^{1/2} - 104.7 \left(\frac{a}{W} \right)^{3/2} + 369.9 \left(\frac{a}{W} \right)^{5/2} - 573.8 \left(\frac{a}{W} \right)^{7/2} + 360.5 \left(\frac{a}{W} \right)^{9/2} \right\}$$